



DBIA

Ground Improvement Technologies for Stabilization and Releveling

**HAYWARD
BAKER**
Geotechnical Construction
KELLER

www.HaywardBaker.com

Before We Get Too Far today lets take a SAFETY Moment



Think Safe, Work Safe, Go Home Safe

CDOT mourners note job's dangers

By Tom McGhee

The Denver Post

POSTED: 04/09/2008 01:00:00 AM MDT

[ADD A COMMENT](#)

Paul Forster and Eladio Lopez should have returned to their families after a day on the job behind a string of orange safety cones four years ago. Instead, a drunken student going the wrong way on Interstate 25 mowed them down, cutting short their lives as they worked to make the highway safer.

They were "out there making a living fixing the roads for the people of Colorado," said Joyce Bunkers, a friend of Forster's who spoke at a ceremony Tuesday honoring Colorado Department of Transportation workers killed on the job.

Since 1929, 57 CDOT employees have died in the line of duty.



Orange carnations are placed Tuesday at the memorial for Colorado Department of Transportation workers killed in the line of duty. The white marble monument, with workers' names, sits in front of CDOT headquarters in Denver. (Kathryn Scott Osler, The Denver Post)



Treating the Symptoms - roadway

- ◆ Void Filling
- ◆ Slabjacking
- ◆ Remove and replace
- ◆ Overlay
- ◆ Monitor

Slabjacking / Void fill

- ◆ Process by which a mobile material is injected between the soil subgrade and the underside of the slab to restore intimate contact and to fill voids. Slabs and sleepers can be leveled to fine tolerances with this process
 - Cement Slurry – not used as often anymore
 - Polyurethane / foam - used most frequently







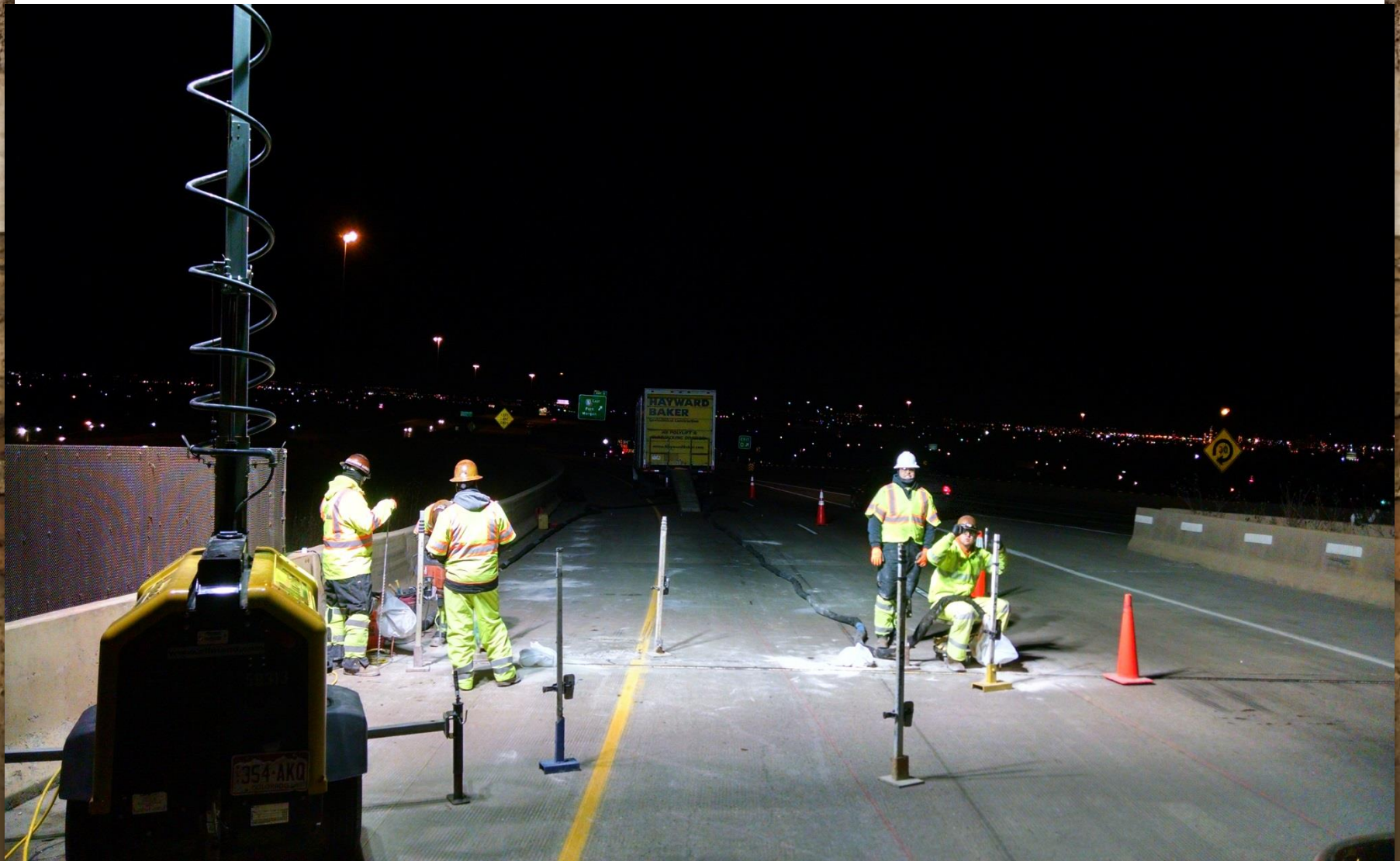


THINK SAFE

Sleeper slabs can be jacked – with caution



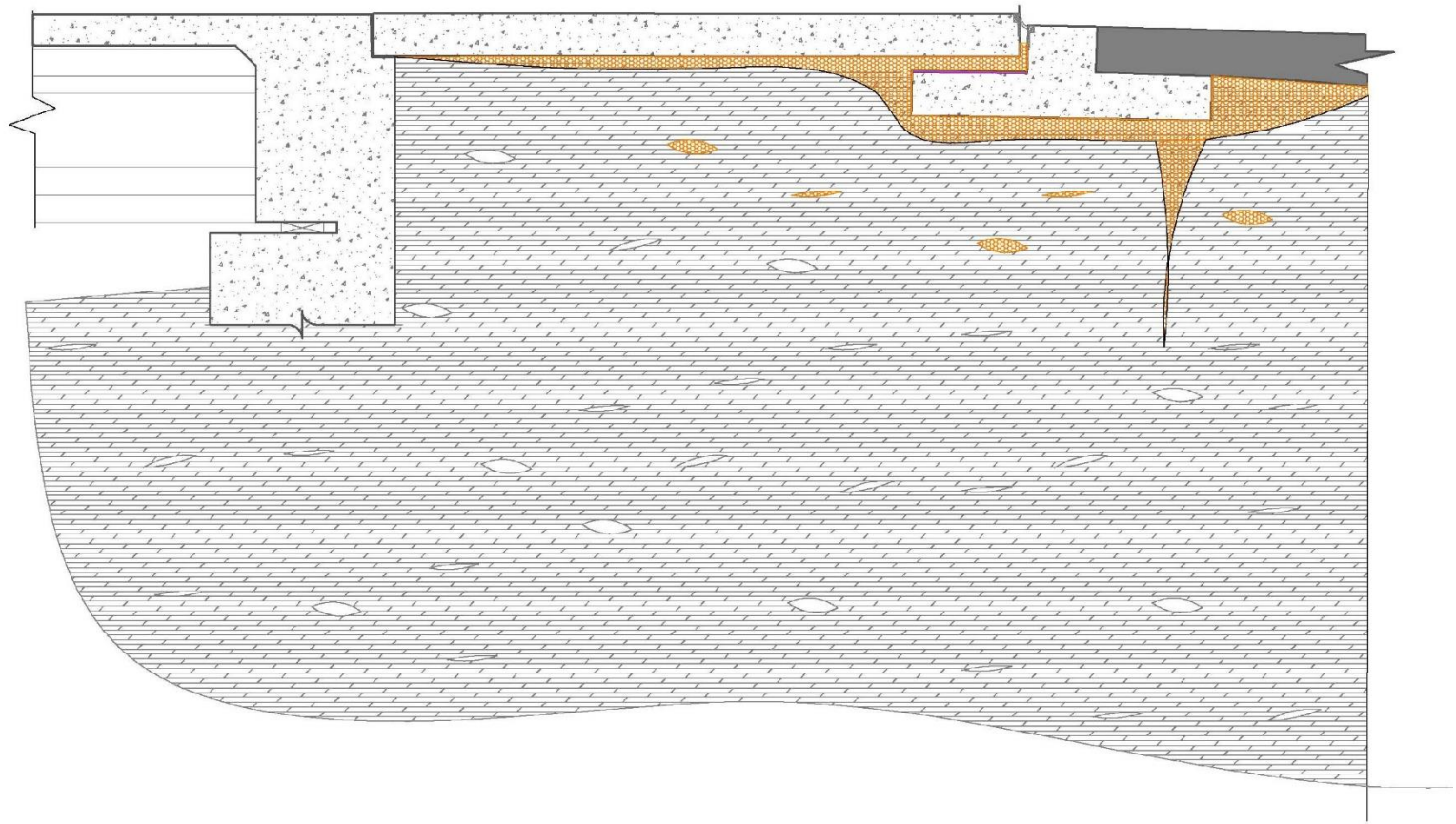
Sleeper and approach releveling



View... Settlement



If using mobile grouts (cement or poly) care must be taken to assure product doesn't get between the slab and sleeper

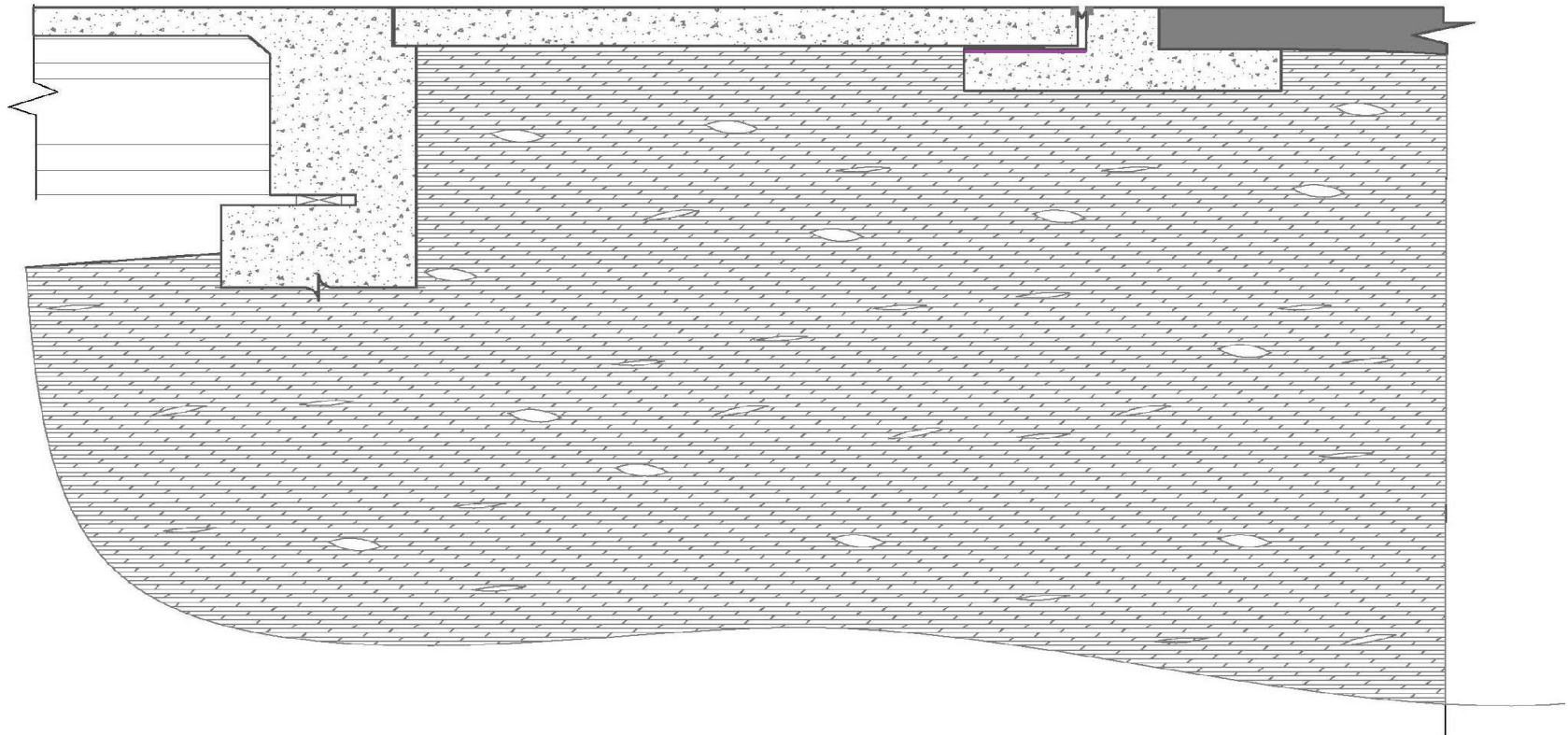


Jacking Sleepers

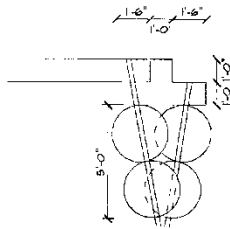
- ◆ Injected in liquid form so can travel some before setting. When trying to lift sleepers, numerous contractors have gotten foam between sleeper and slab and lifted slab off the sleeper.
 - Big profile dip is often eliminated, but sharp bump at sleeper is there forever if slabs are picked up off of sleeper.
 - Reports of sleepers being twisted
 - It is a chemical reaction and things can go awry

Things to consider

If the consolidation or settlement problem is deep and ongoing, settlement will likely continue regardless of which material is injected at the shallow depths.



How can one reduce risk of poly (or slurry) infiltration in between sleeper and slab while assuring controlled lift?

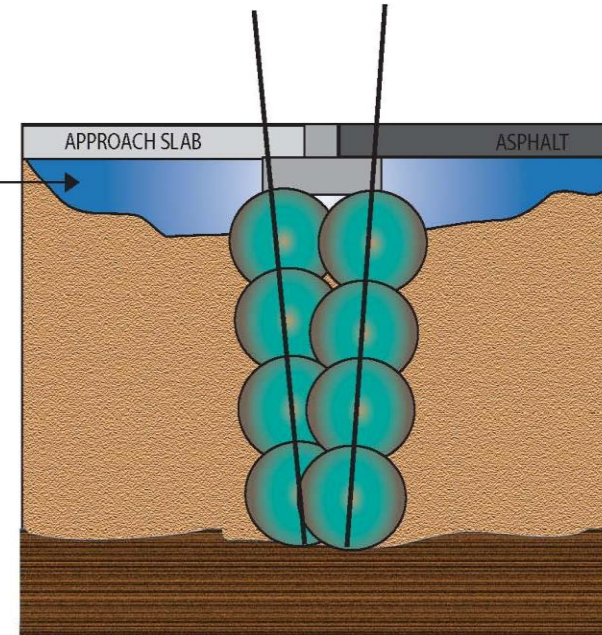


SLEEPER RELEVELING PROCESS

- STEP 1 - OWNER OR G.C. PROVIDES TARGET ELEVATIONS ON SLAB.
- STEP 2 - HBI DRILLS NOMINAL 3" HOLES THROUGH SLEEPER ON APPROX. 5' CENTERS.
- STEP 3 - HBI INSTALLS GROUT CASING TO DEPTH OF APPROX. 3'-5'.
- STEP 4 - A LOW MOBILITY GROUT IS INJECTED TO DENSIFY, STRENGTHEN AND COMPACT SHALLOW ZONE BELOW SLEEPER. MULTIPLE POINTS ARE INJECTED CONCURRENTLY WHILE RELEVELING SLEEPER TO DESIRED ELEVATION.
- STEP 5 - UPON COMPLETION OF SLEEPER RELEVELING, SMALL DIAMETER HOLES SHALL BE DRILLED AS NEEDED THROUGH THE ADJACENT SLABS TO FILL VOIDS AND RELEVEL SLABS.
- STEP 6 - UPON COMPLETION OF ALL GROUTING, ALL HOLES SHALL BE PATCHED WITH A NONSHRINK GROUT.

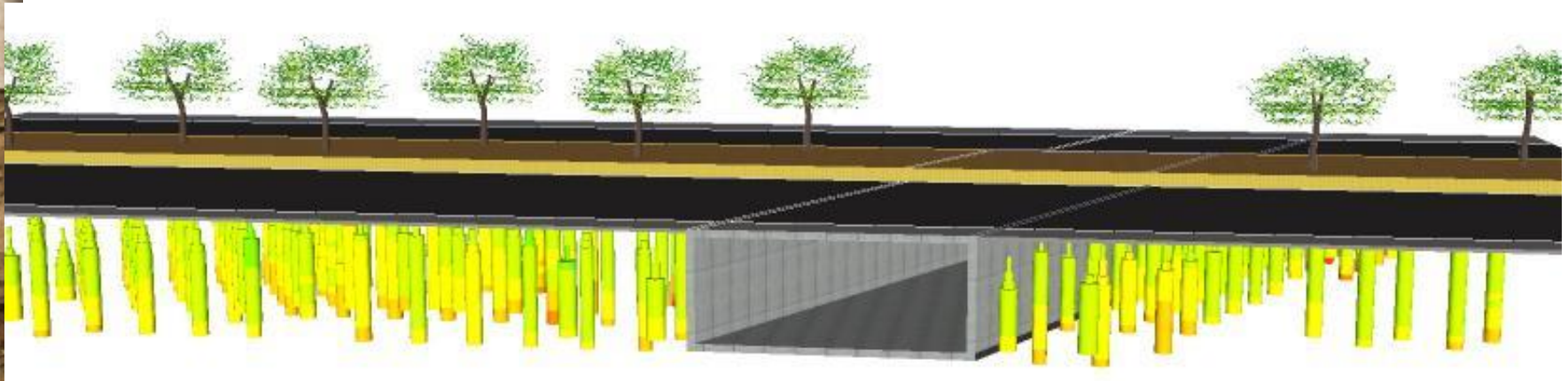
LMG / HB Polylift

POLYURETHANE
FOAM
INJECTION TO
FILL VOIDS

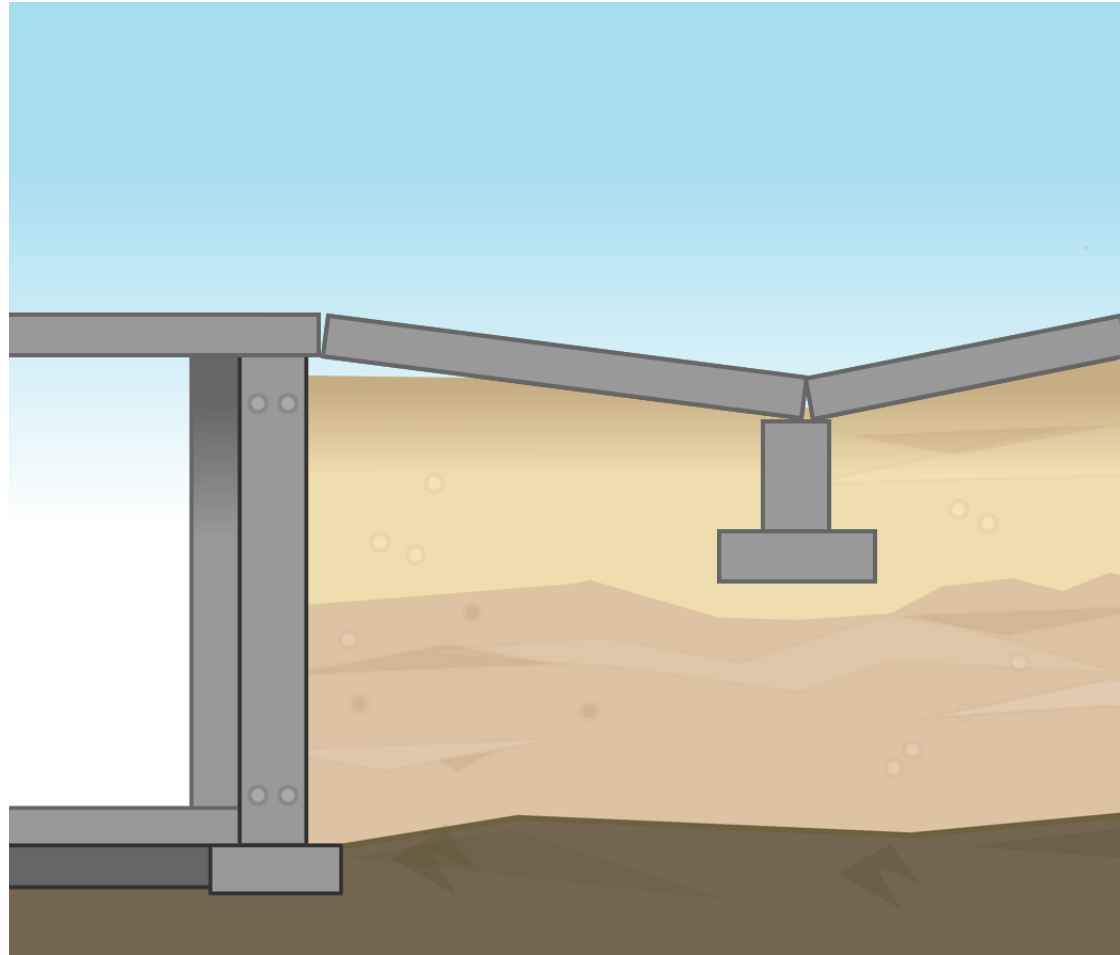


Introduction-- What is Compaction Grouting or LMG

Compaction Grouting uses displacement to improve ground conditions. A very viscous (low-mobility), aggregate grout is pumped in stages, forming grout bulbs, which displace and densify the surrounding soils. Is also a very controlled means of releveling



Bridge Approach Remediation: LMG controlled grouting and stabilization



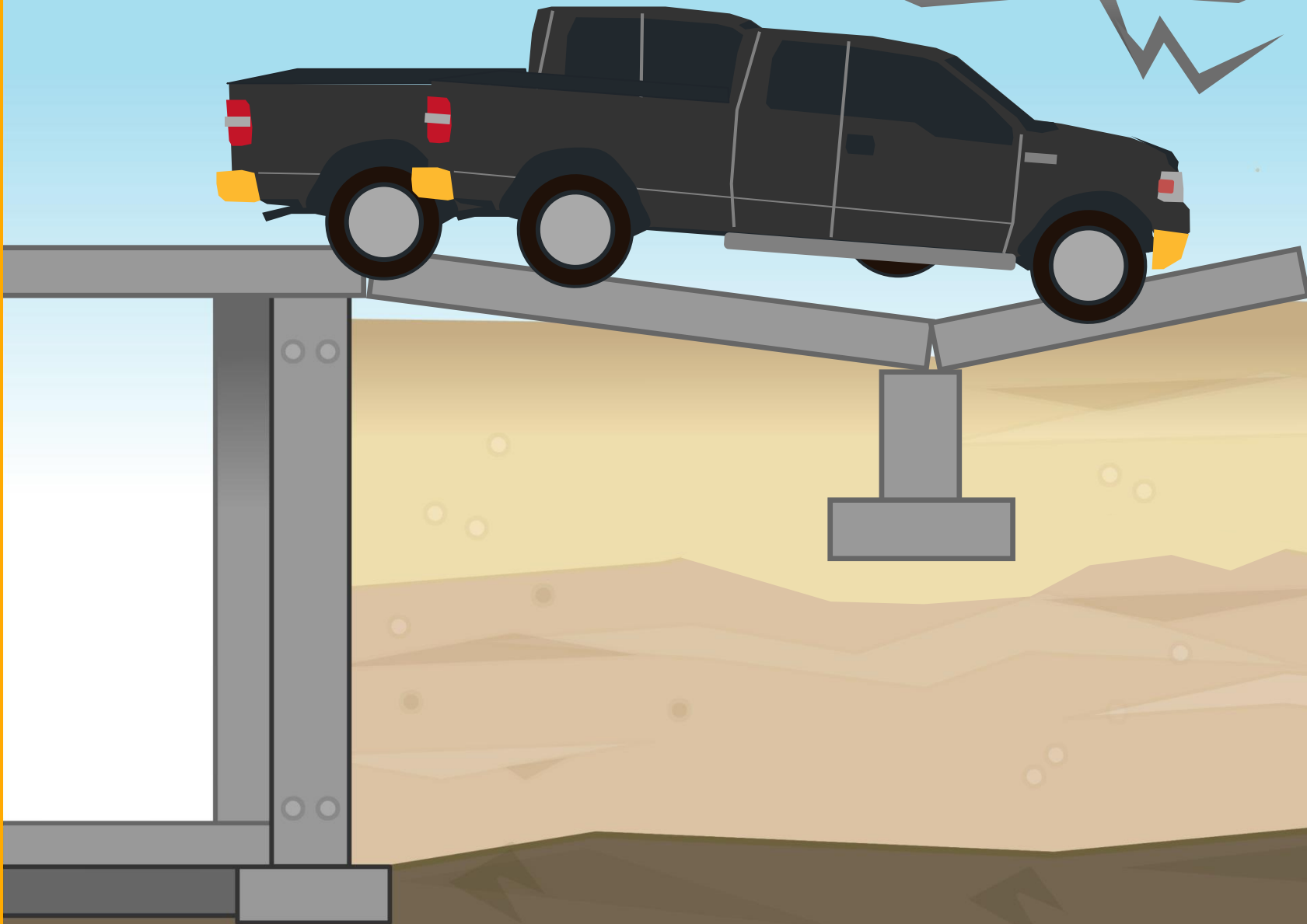
Low Mobility Grouting / Poly Combo Bridge Approach Remediation

Can be used to treat both the “symptom and the cause” or as a shallow to moderate depth controlled lifting process--- LMG Grouting is not outdated, a bad thing, nor is it going away.

Click to begin demonstration!

Problem...

!!!!

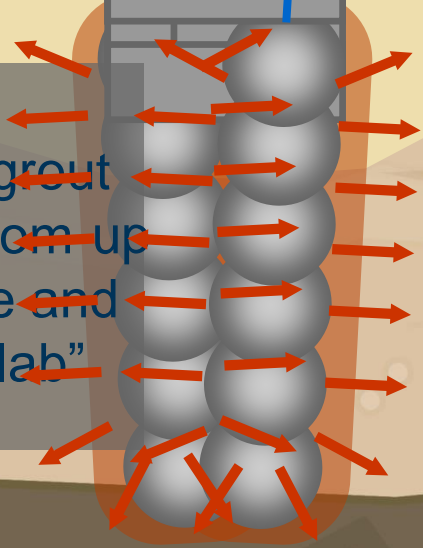


Step 3- Utilization of HB PolyLift™ Polyurethane Foam Injection to fill voids and re-level adjacent slabs.



Grout Installation: ("bottom-up method")

Step 2- Install compaction grout columns via the "bottom up method," to stabilize and re-level "sleeper slab"

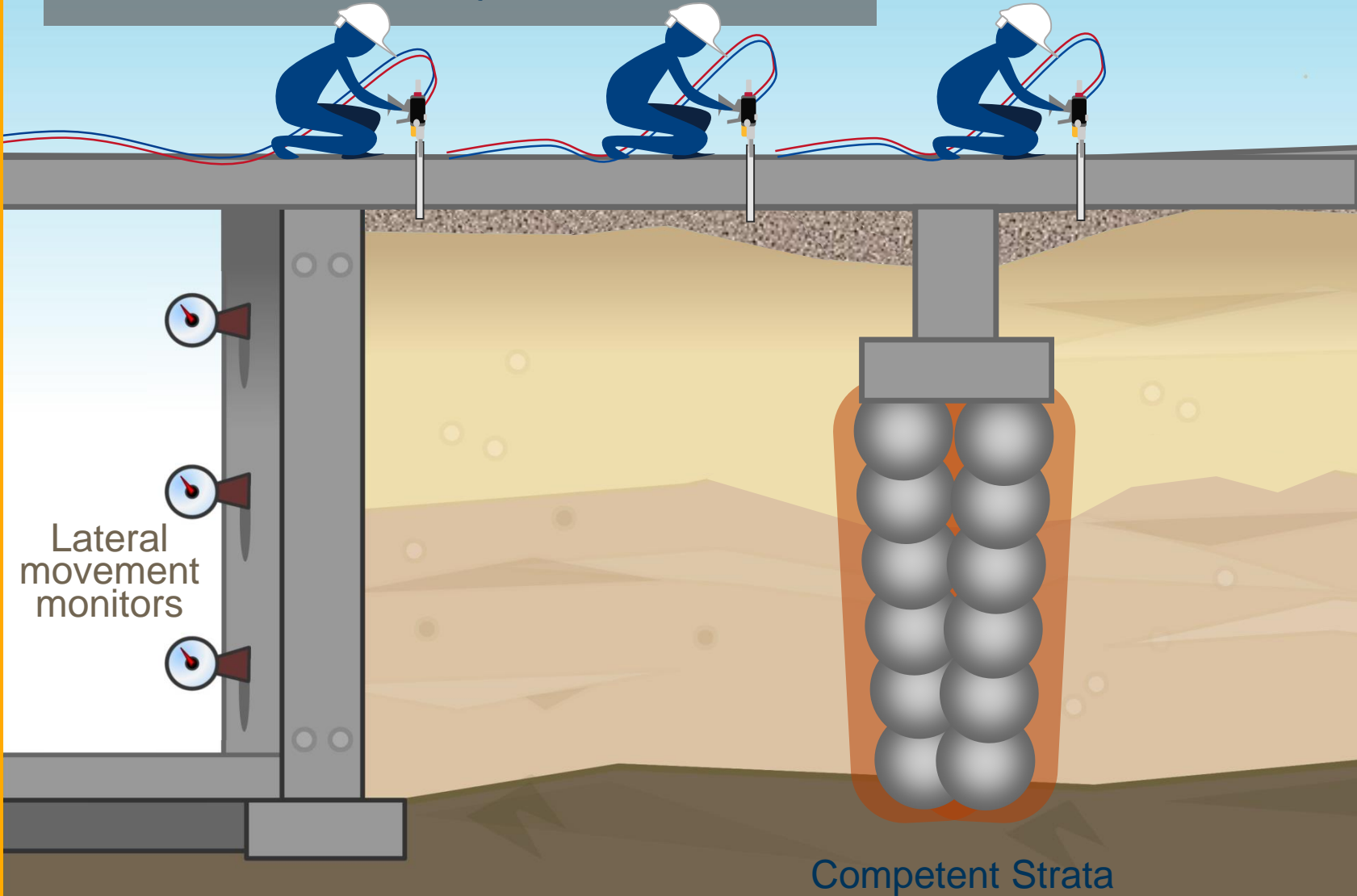


Lateral movement monitors

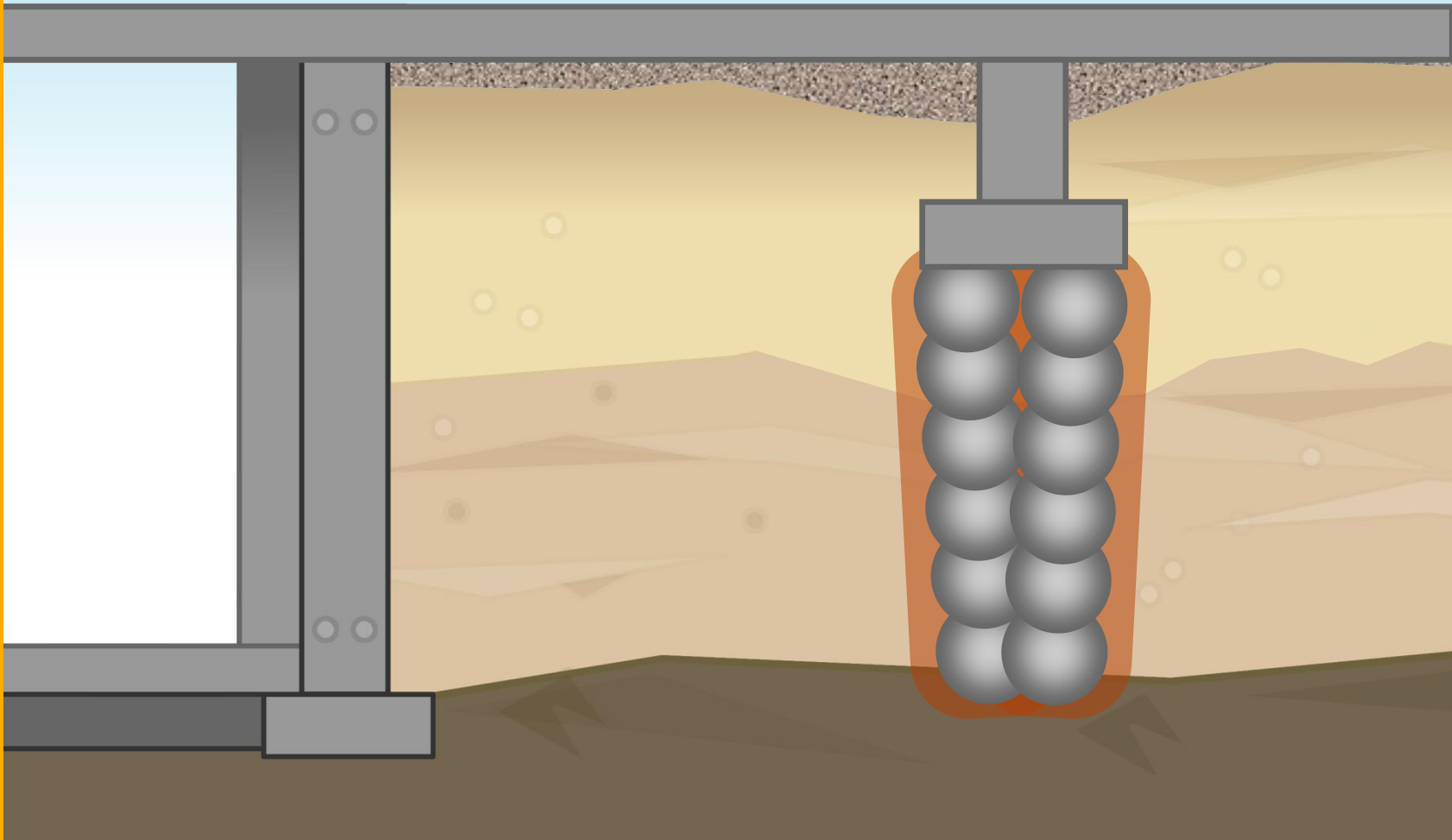


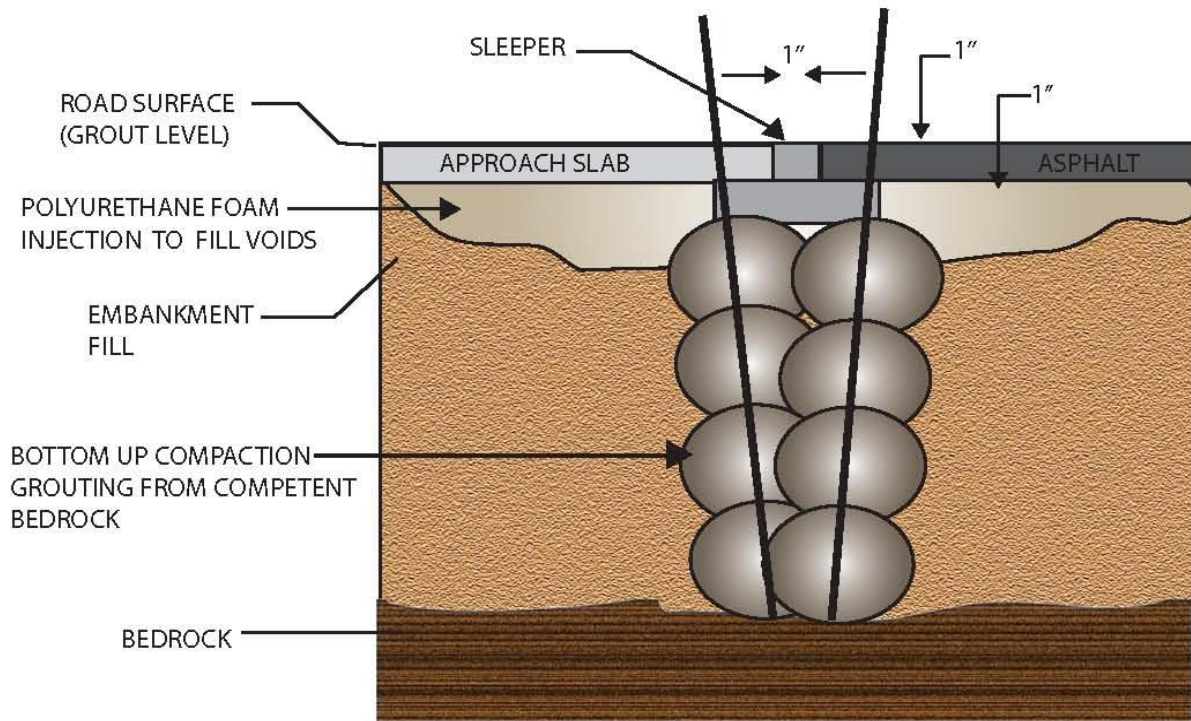
Competent Strata

Step 3- Utilization of HB PolyLift™
Polyurethane Foam Injection to fill voids
and re-level adjacent slabs.

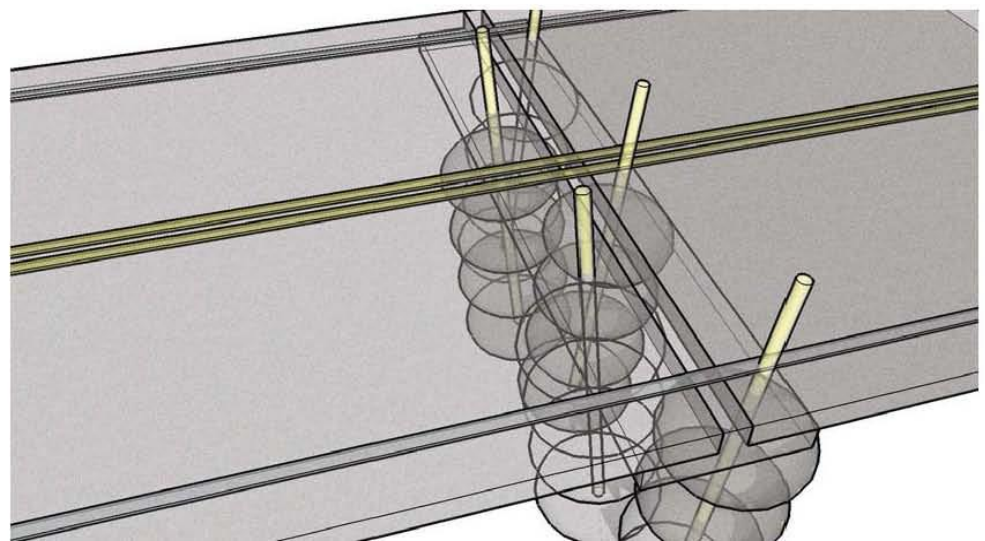
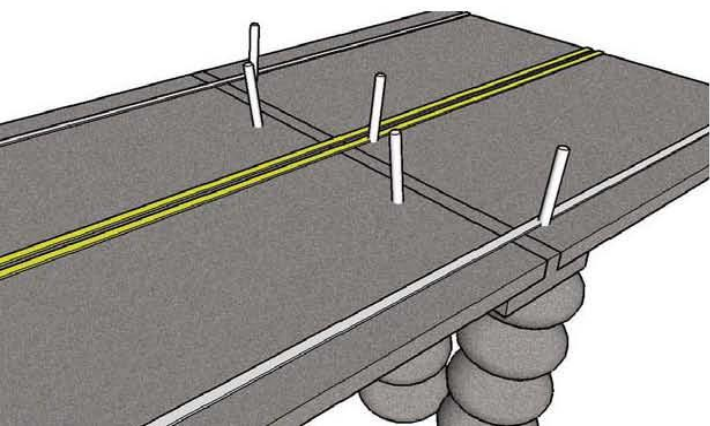


End





SECTION VIEW
 1/4" = 1'0"



T-Rex Bridge Approach stabilization and re-leveling



Bridge Sleeper / approach



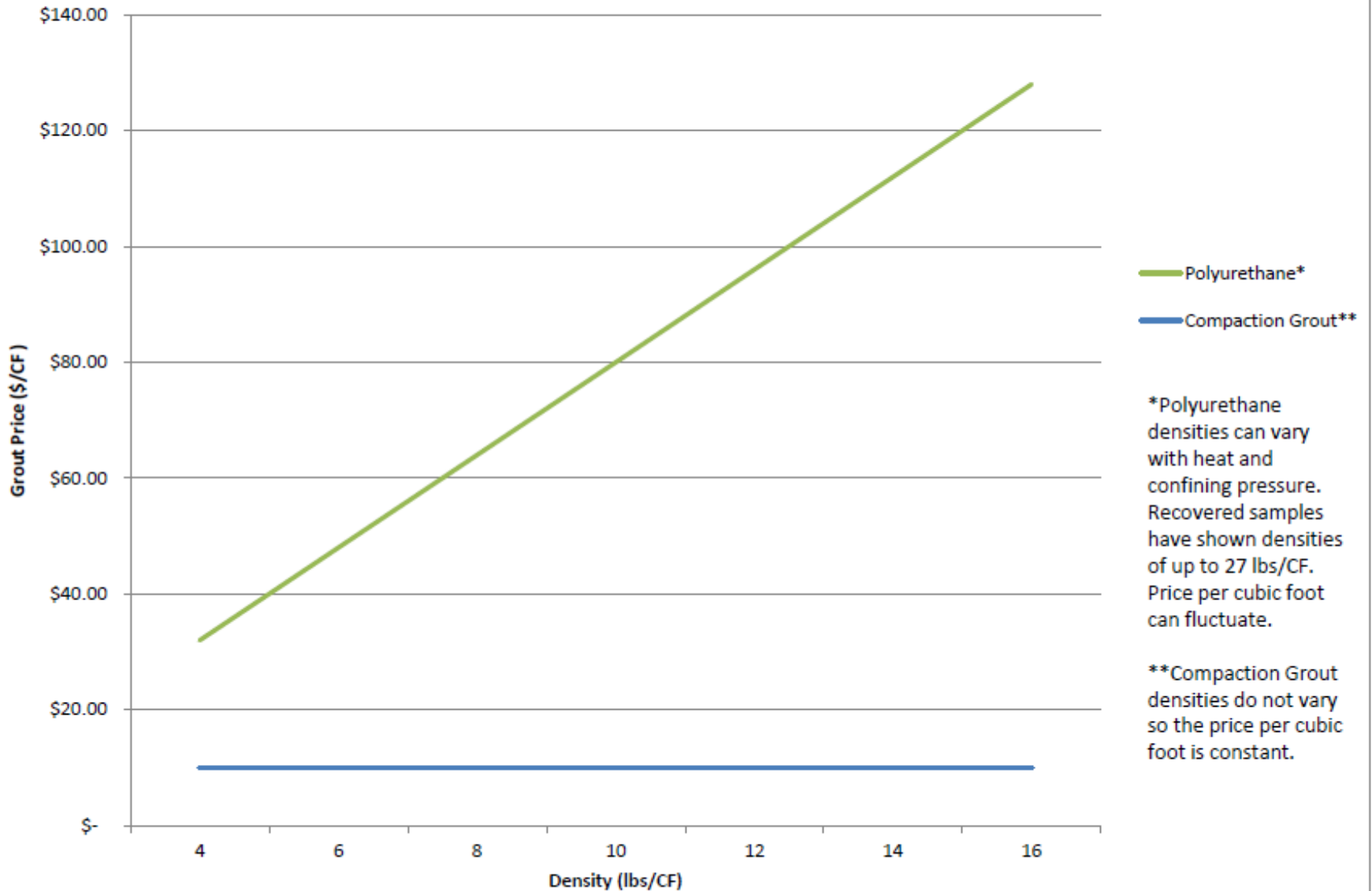
So why utilize the low mobility and Poly lift systems in conjunctions with each other?

- 1) Utilize the time tested, cost effective, predictable efficient low mobility grout to lift load bearing elements such as a sleeper.
- 2) Utilize low mobility grout to densify the subgrade to a competent bearing stratum if desired for long term stability.
- 3) Utilize lightweight poly foam to fill voids and to lift adjacent slabs in conjunction with the low mobility grout
 - * we don't ask the "foam" to do too much. Anytime you can use foam under the least amount of pressure, you get a "better bang for your buck".
- 4) Overall cost – This combination process is much more cost effective
- 5) Long term stability – Combination of the processes provides a nice balance of cost and durability
- 6) Predictable results
- 7) Risk reduction in getting foam between sleeper and slab

Potential Advantages of Compaction or LMG in roadway or bridge work

- ◆ Cost – still very economical – (density doesn't change when injected so you get exactly what you think you are paying for)
- ◆ Readily available
- ◆ Time tested and proven
- ◆ Very controlled and predictable process for re-leveling sleepers
- ◆ Pin point accuracy
- ◆ Assured and warranted long term solution if full treatment depth is performed

Price of Grout vs Density



Things to consider

- ◆ When grouting directly adjacent to MSE walls, embankments or abutments, one must monitor and grout with caution as the densification process compacts soils for some distance from the point of injection as determined by grout placement quantity
 - (Sequencing is very important)
 - (admittedly, it may not always be the right in-situ remedial technique)

Other uses of Compaction or Low Mobility Grouts

- ◆ Embankment Fill rehab
- ◆ MSE Wall stabilization
- ◆ Wall Backfill rehab
- ◆ Trench Backfill rehab
- ◆ Compensation / Emergency grouting (tunneling gone awry, utility break)

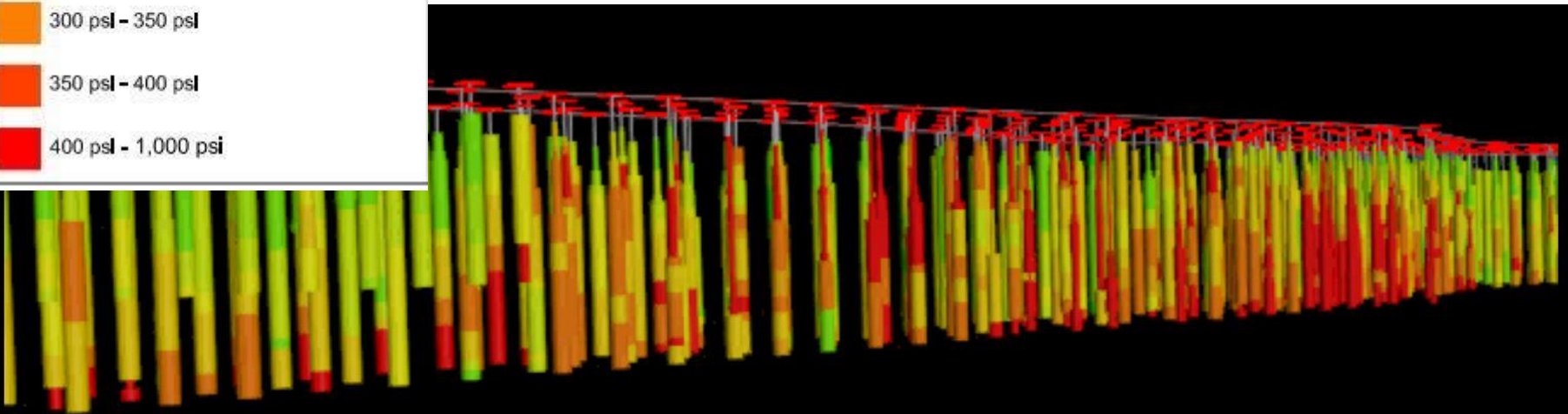
Man Placed Fill around box culvert rehab



3D As Builts are now commonplace

LEGEND

GROUT PRESSURES

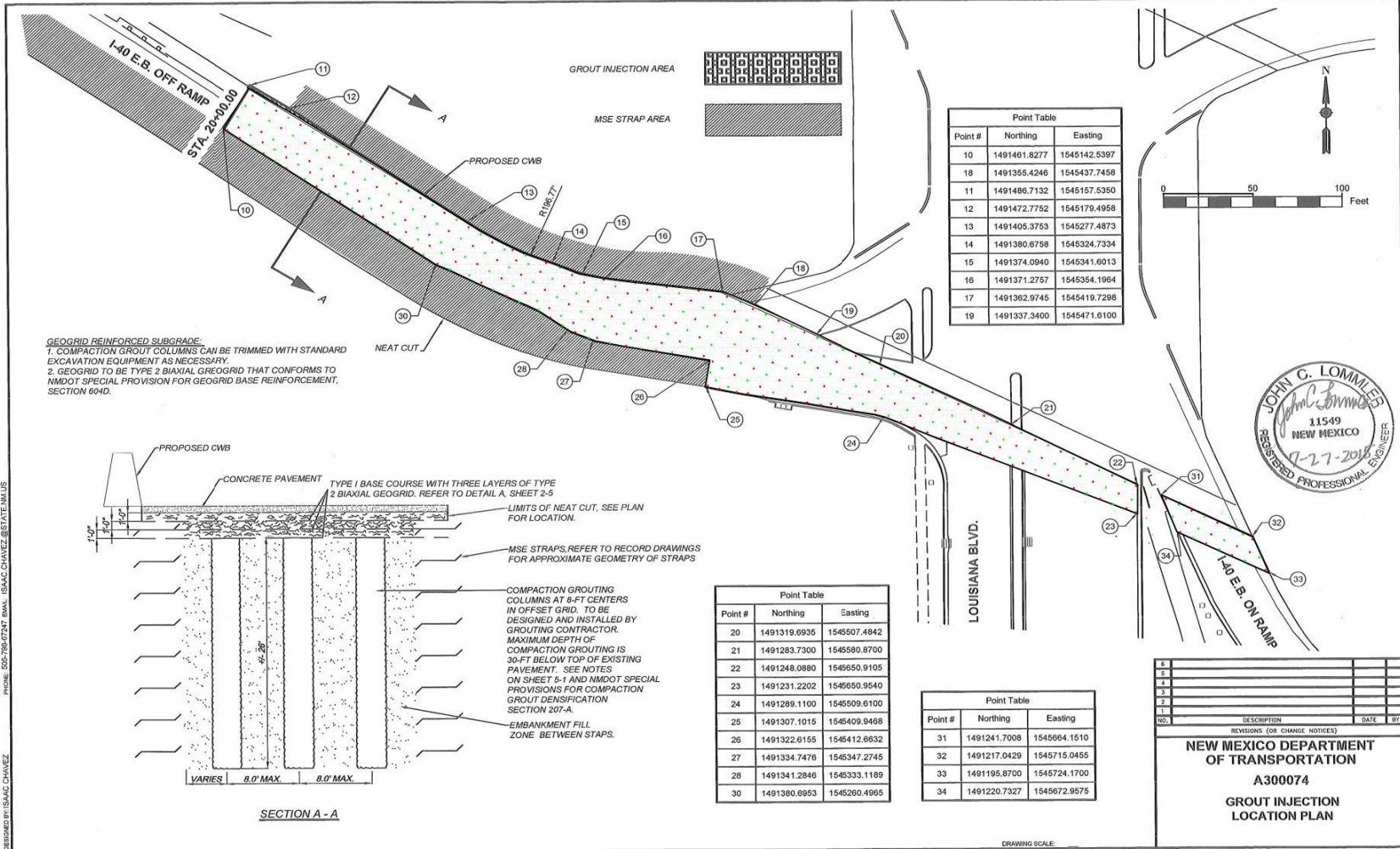


Embankment Fill rehab between MSE Walls

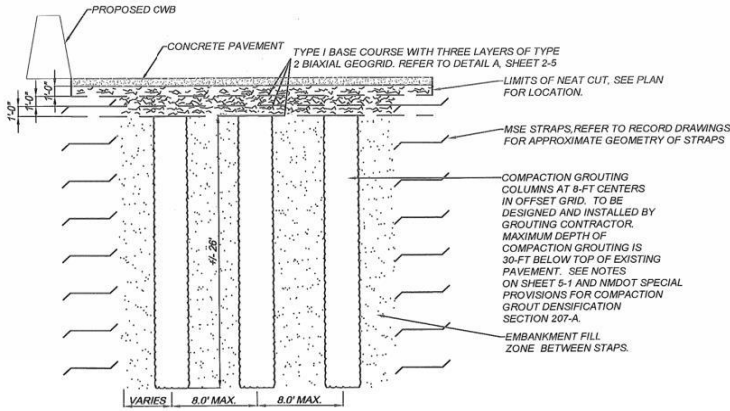


Grouting Plan

PROJECT CONTROL NUMBER: A300074



GEOGRID REINFORCED SUBGRADE:
 1. COMPACTION GROUT COLUMNS CAN BE TRIMMED WITH STANDARD EXCAVATION EQUIPMENT AS NECESSARY.
 2. GEOGRID TO BE TYPE 2 BIAIXIAL GEOGRID THAT CONFORMS TO NMDOT SPECIAL PROVISION FOR GEOGRID BASE REINFORCEMENT, SECTION 604D.



SECTION A - A

Point Table		
Point #	Northing	Easting
10	1491461.8277	1545142.5397
18	1491355.4246	1545437.7458
11	1491486.7132	1545157.5350
12	1491472.7752	1545179.4958
13	1491405.3753	1545277.4873
14	1491380.6758	1545324.7334
15	1491374.0940	1545341.6013
16	1491371.2757	1545354.1964
17	1491362.9745	1545419.7298
19	1491337.3400	1545471.6100

Point Table		
Point #	Northing	Easting
20	1491319.6935	1545507.4842
21	1491283.7300	1545580.8700
22	1491248.0880	1545650.9105
23	1491231.2202	1545650.9540
24	1491289.1100	1545509.6100
25	1491307.1015	1545409.9468
26	1491322.6156	1545412.6632
27	1491334.7476	1545347.2745
28	1491341.2846	1545333.1189
30	1491380.0953	1545260.4965

Point Table		
Point #	Northing	Easting
31	1491241.7008	1545664.1510
32	1491217.0429	1545715.0455
33	1491195.8700	1545724.1700
34	1491220.7327	1545672.9575



NO.	DESCRIPTION	DATE	BY

NEW MEXICO DEPARTMENT OF TRANSPORTATION
A300074
GROUT INJECTION LOCATION PLAN

DESIGNED BY ISAAC CHAVEZ
 PHONE: 505-766-6747 EMAIL: ISAAC.CHAVEZ@STATE.NM.US

DRAWING SCALE:
 NEW MEXICO PROJECT NO. A300074

SHEET NO. 5-2

Grouting Operation



Actual Field Log

HAYWARD BAKER
A Keller Company

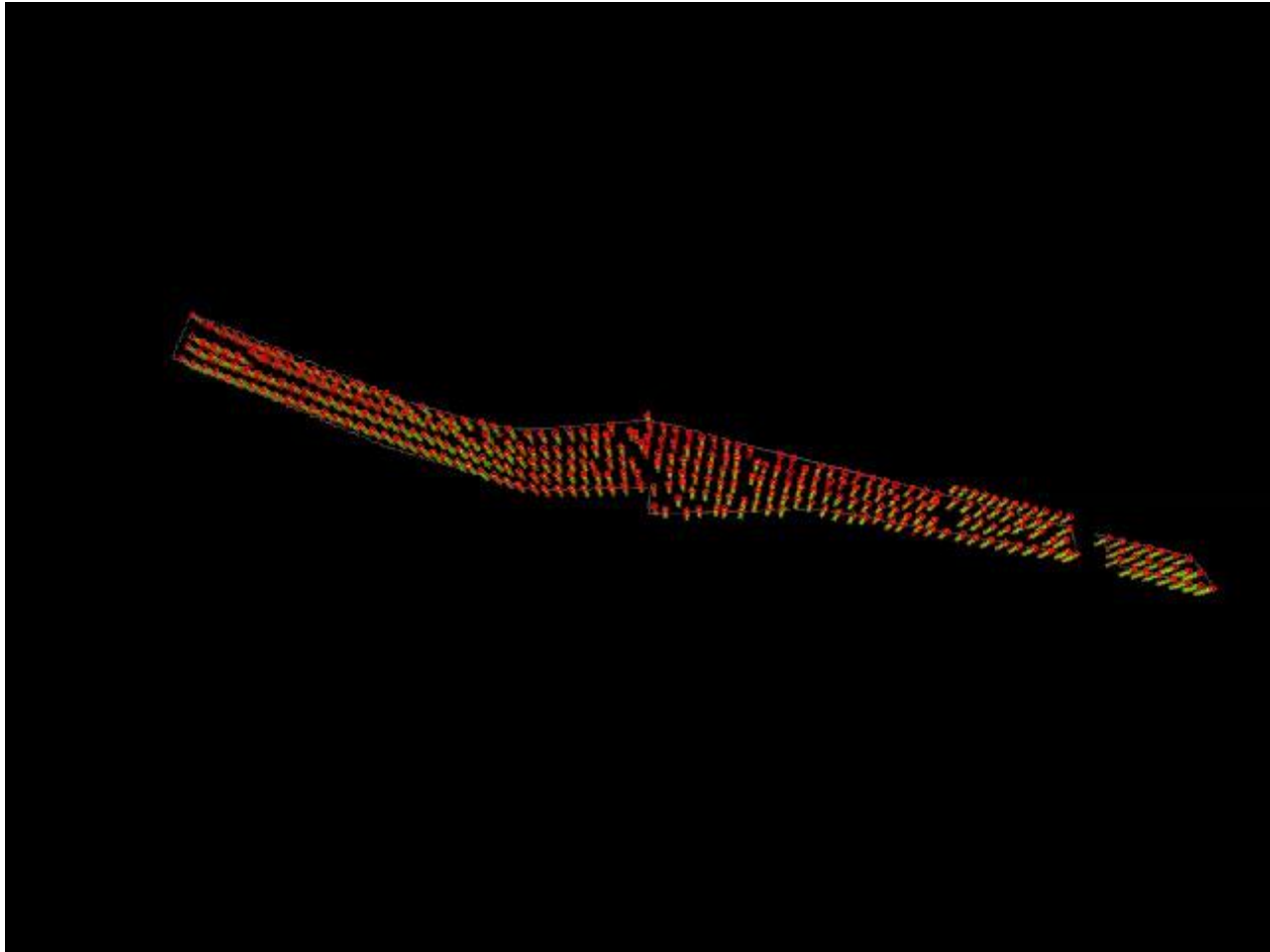
KELLER

COMPACTION GROUT LOG

NO. PROJECT NO. 600512 PAGE _____ OF _____
 PROJECT NAME J-40 DATE 3-6-16
 GROUT POINT NO. B-94 PUMP TYPE _____
 MAXIMUM DEPTH _____ PUMP CAPACITY (C.F./STROKE) _____
 GROUT TECHNICIAN Richie M PUMP RATE _____

DEPTH (FEET)	TIME		MAXIMUM GAGE PRESSURE (PSI)	GROUT QUANTITY		SURFACE HEAVE (IN)	COMMENTS
	START	STOP		STROKES	CU. FT.		
30			250				
29			250				
28			250				
27			250				
26			250				
25			250				
24			250				
23			250				
22			250				
21			250				
20			250				
19			250				
18			250				
17			250				
16			250				
15			250				WALL MOVEMENT
14			250				
13			250				
12			250				WALL MOVEMENT
11			250				
10			250				
9			250				
8			250				
7			250				
6			250				
5			250				
4			250				
			250	TOTALS	2		
			250		0.5		grout volume
			250		0.5		

AMOUNT QUANTITY THIS PAGE 67 250 (C.F.)
 C.F. Y. HAYWARD BAKER SUPERINTENDENT _____
 COMPLETION TIME _____ CLIENT'S REPRESENTATIVE _____



Tablet Compaction Software

Hayward Baker Compaction Grouting Tablet

File View Edit Help

COMPACTION GROUT LOG

HBI PROJECT NO.: 15881 DATE: 6/3/2010

PROJECT NAME: Lockport Canal Walls PUMP TYPE: P-1

GROUT POINT NO.: Sample PUMP STROKE VOL.: 1.0 cf

MAXIMUM DEPTH: 100.0 ft GROUT TECHNICIAN: JRB

STAGE LENGTH: 1.0 ft

REFUSAL CRITERIA:

- TARGET VOLUME: 9.0 cf
- TARGET PRESSURE: 200.0 psi
- MAX PRESSURE: 400.0 psi

DEPTH: Manual GAUGE PRESSURE: 369.33 psi STROKES: 30 STAGE VOLUME: 30.0 cf TOTAL VOLUME: 69.0 cf ELAPSED TIME: 00:01:08

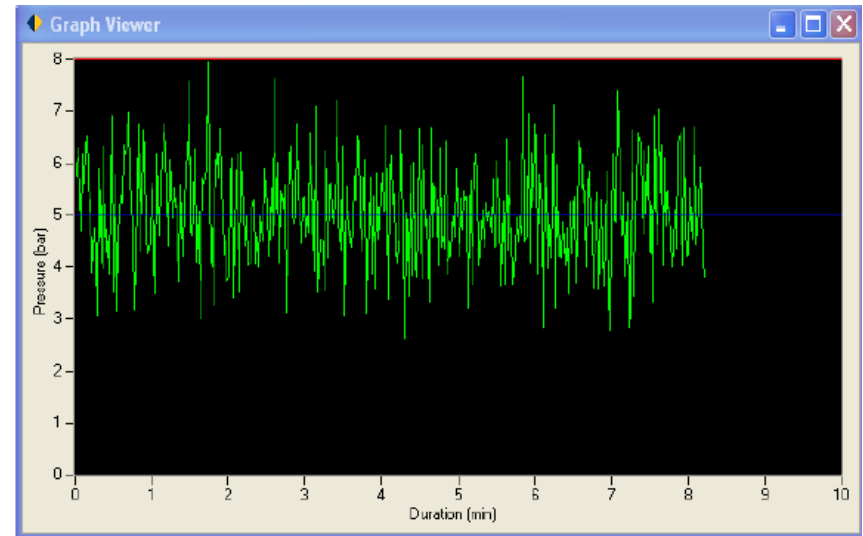
Stop | Next Stage

START DEPTH	END DEPTH	START TIME	STOP TIME	DURATION	MAX GAUGE PRESSURE	MAX HEADER PRESSURE	STROKE COUNT	GROUT VOLUME	COMMENTS
100.0	99.0	1:55:40 PM	1:55:45 PM	00:00:05	668.18	368.18	5	5.0	
99.0	98.0	1:55:46 PM	1:55:47 PM	00:00:01	668.18	368.18	2	2.0	
98.0	97.0	1:55:47 PM	1:55:49 PM	00:00:03	668.18	368.18	1	1.0	
97.0	96.0	1:55:49 PM	1:55:50 PM	00:00:00	0.00	0.00	1	1.0	
96.0	95.0	1:55:50 PM	1:55:51 PM	00:00:01	611.81	311.81	0	0.0	
95.0	94.0	1:55:51 PM	1:56:12 PM	00:00:21	721.69	421.69	21	21.0	
94.0	93.0	1:56:12 PM	1:56:18 PM	00:00:06	620.17	320.17	7	7.0	
93.0	92.0	1:56:18 PM	1:56:20 PM	00:00:03	649.21	349.21	2	2.0	
92.0	91.0	1:56:21 PM	1:56:49 PM	00:00:28	723.75	423.75	30	30.0	
91.0	90.0			00:00:00	0.00	0.00	0	0.0	
90.0	89.0			00:00:00	0.00	0.00	0	0.0	
89.0	88.0			00:00:00	0.00	0.00	0	0.0	
88.0	87.0			00:00:00	0.00	0.00	0	0.0	
87.0	86.0			00:00:00	0.00	0.00	0	0.0	

6/3/2010 1:56:48 PM Connection: Connected Logger: On

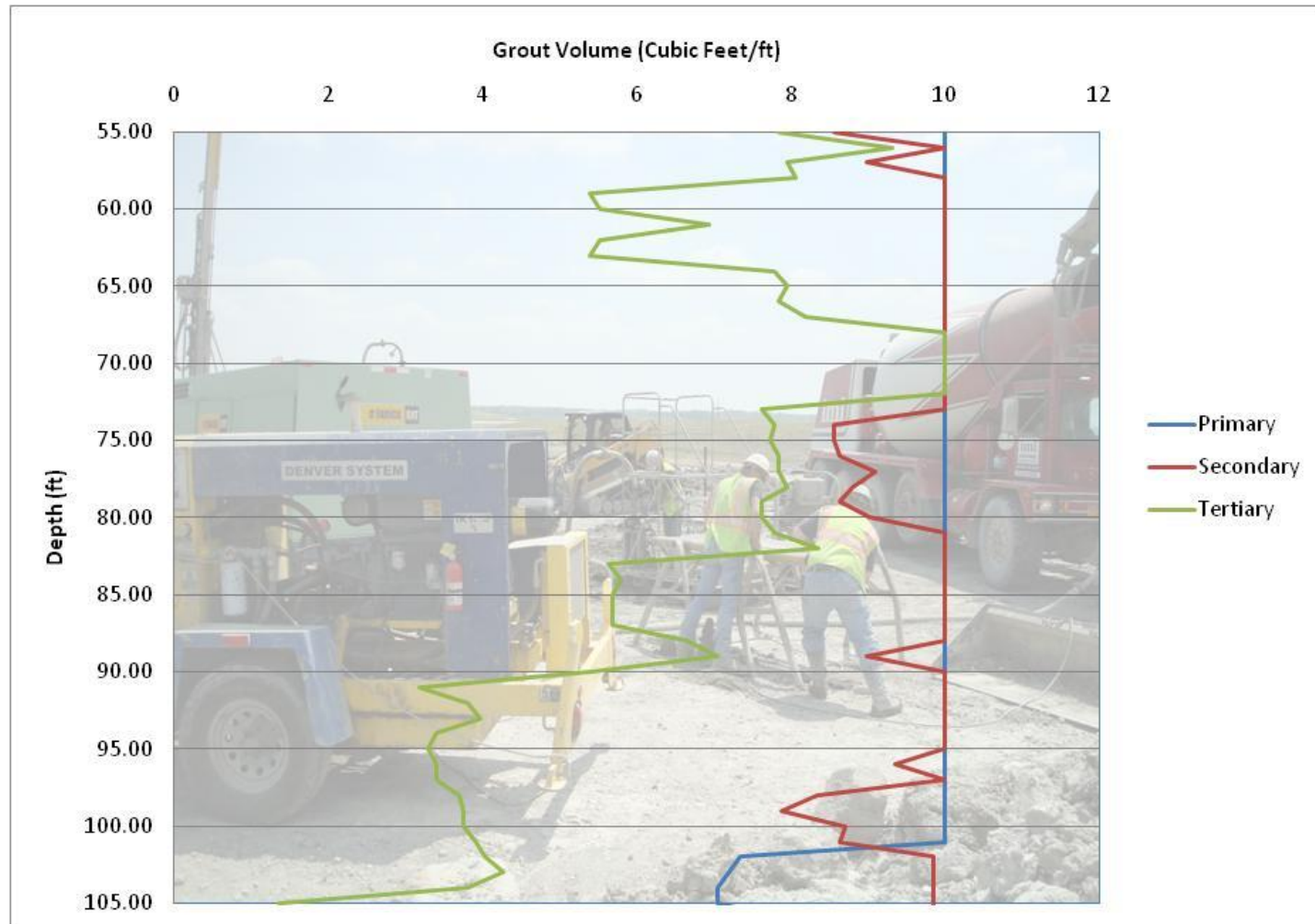
Tablet Compaction Features

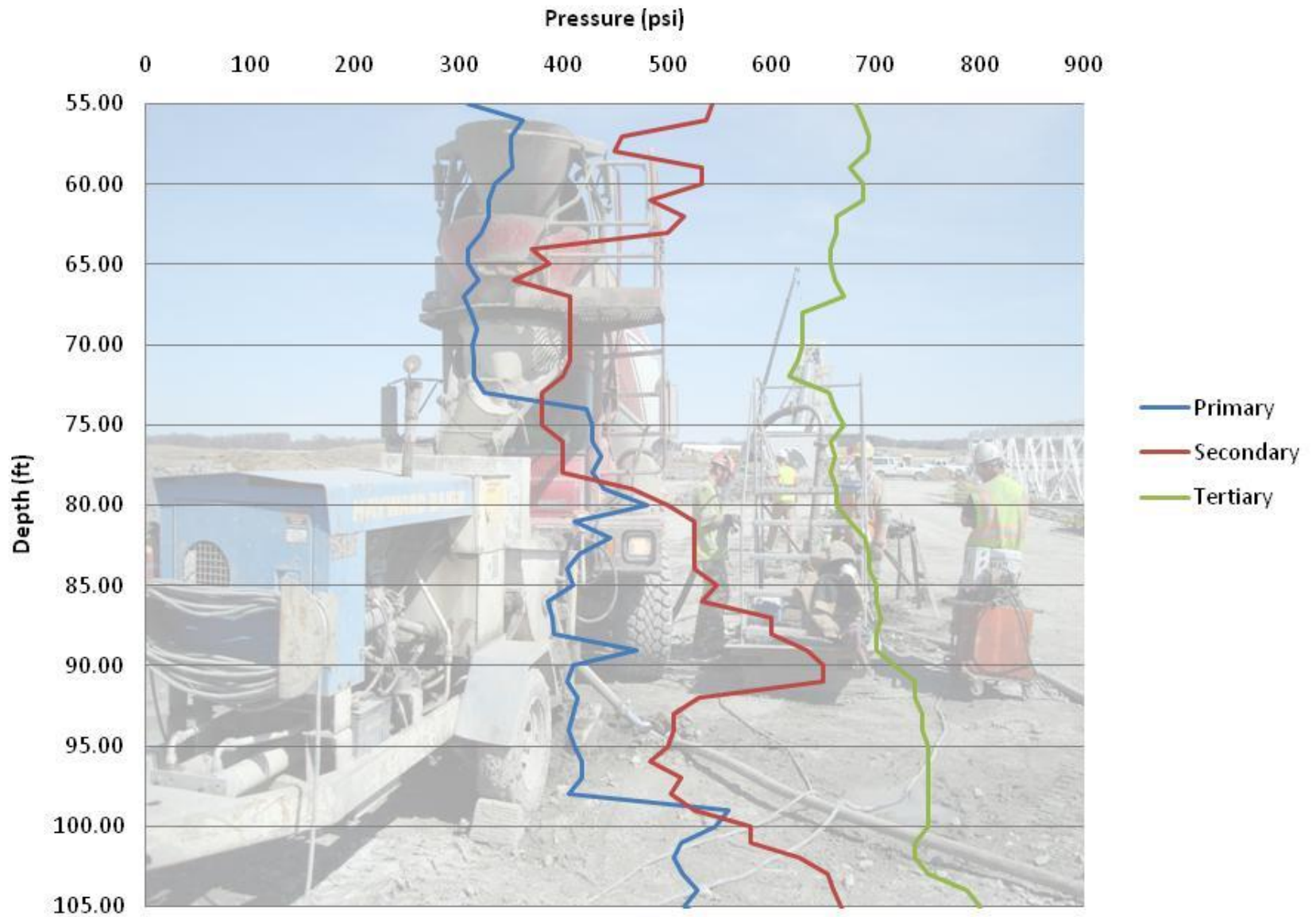
- ◆ Stores data in SQL database on local machine.
- ◆ Syncs data directly to DAQ server database
- ◆ Finished Excel logs are automatically output at the end of each operation



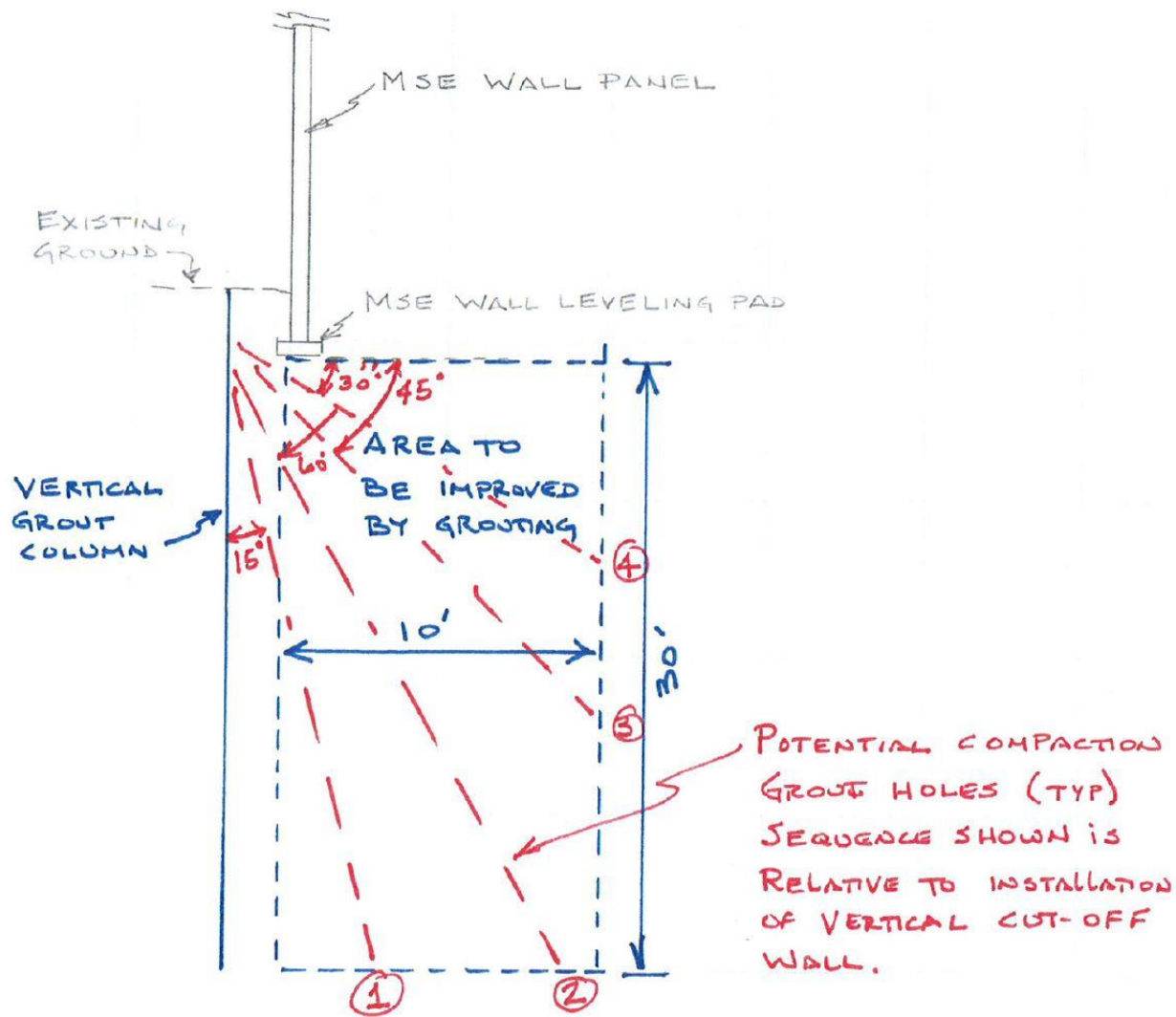
- ◆ **Real-time graphs of pressure, stroke count, volume**
- ◆ **Add comments for each stage**

USING HOLE SEQUENCING FOR VERIFICATION

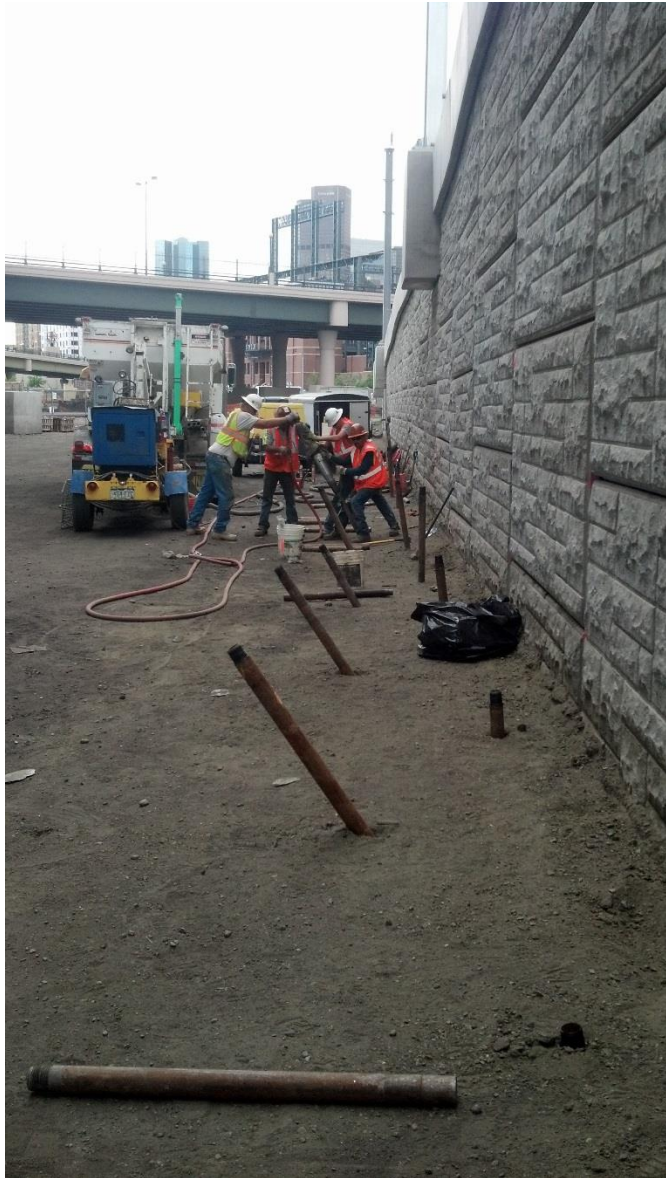


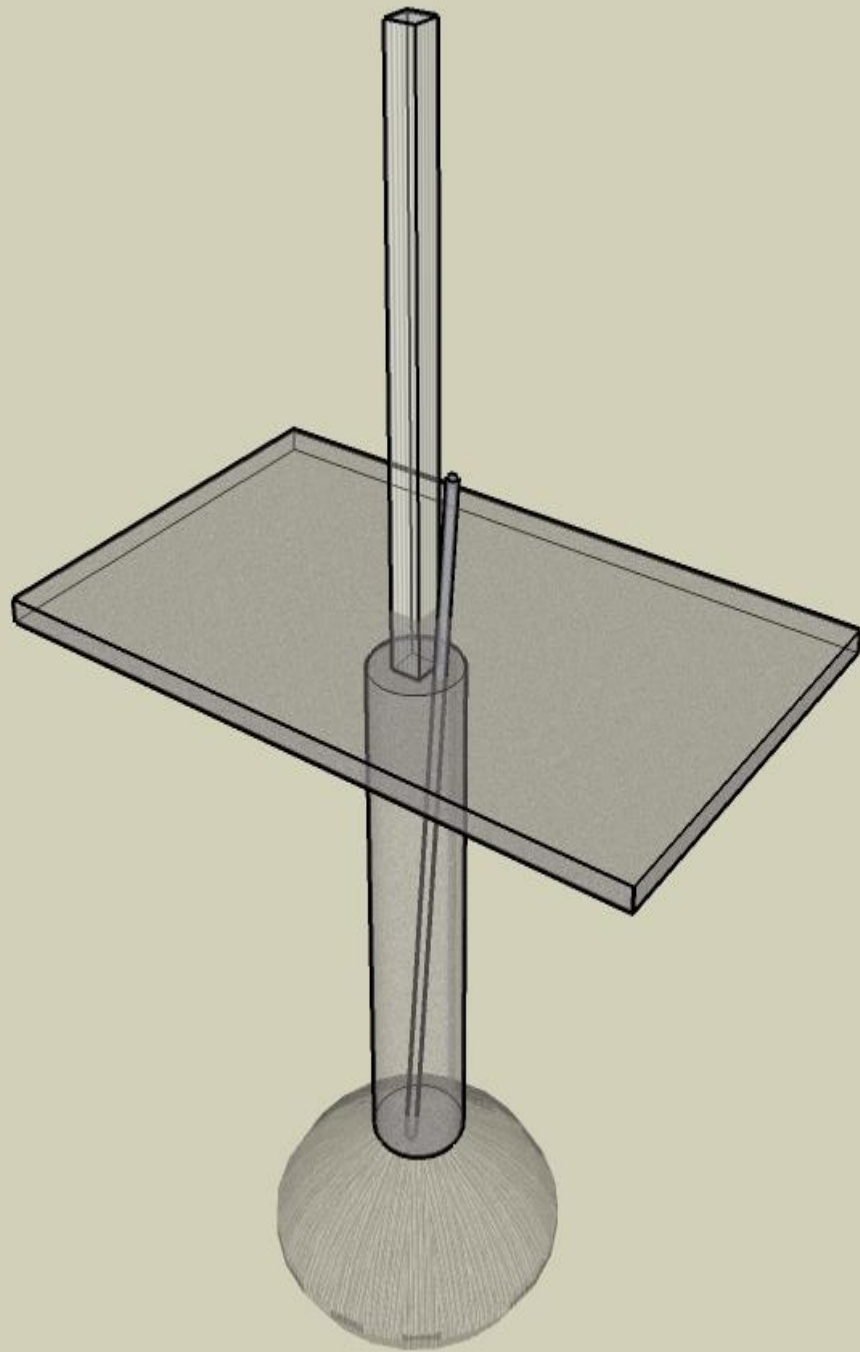


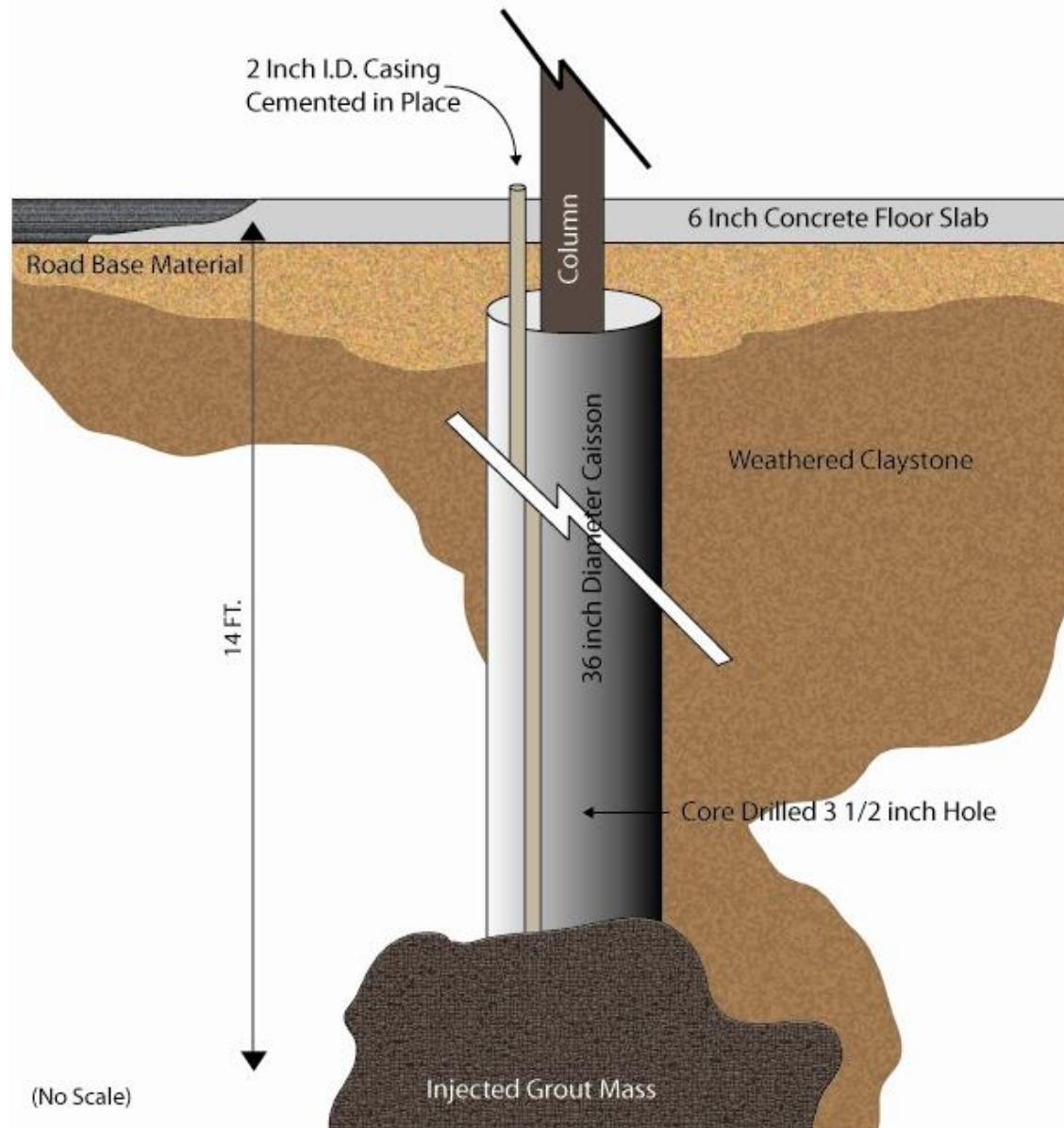
MSE Wall and footing stabilization



MSE / Retaining walls re-leveled







(No Scale)

Compensation Grouting– Tunneling

Mississippi Ave. outfall Tunnel Project Denver, CO

◆ Problem Description:

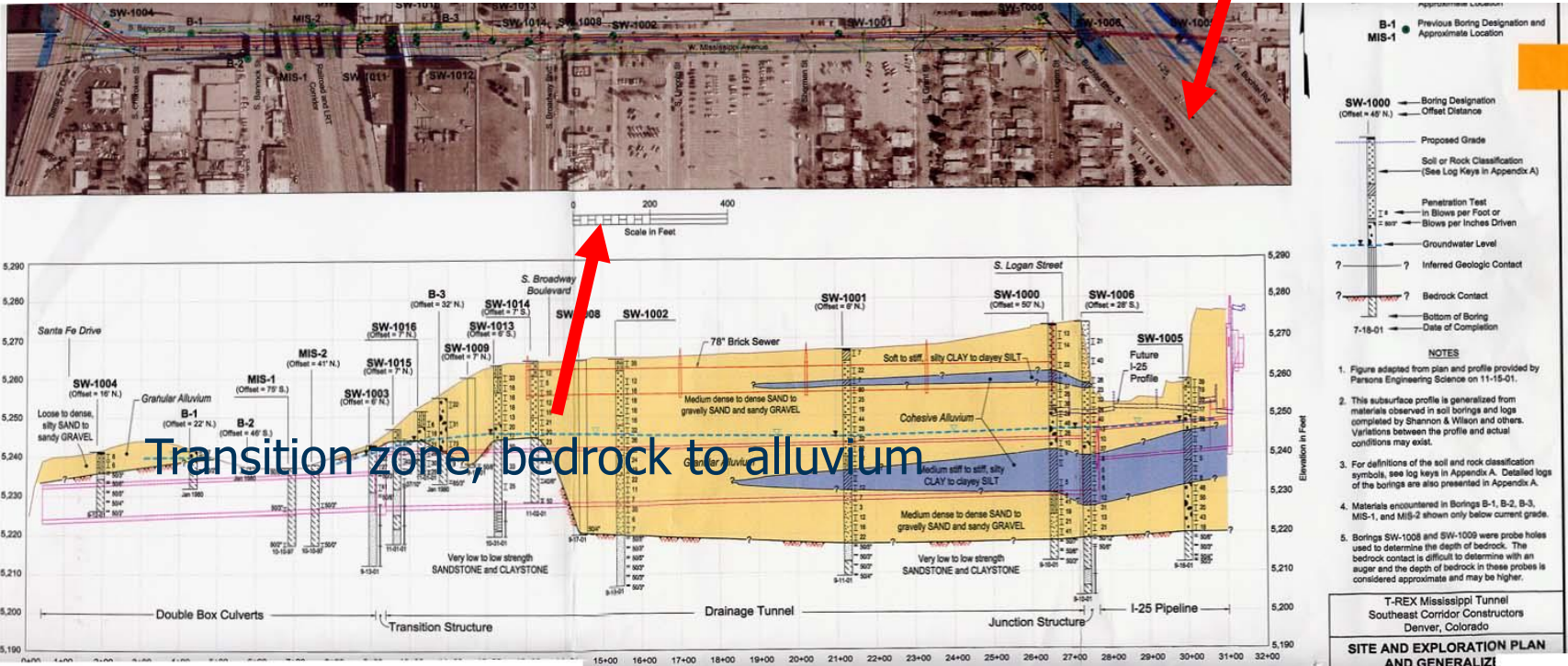
◆ Fifteen-foot diameter TBM cutting in claystone, transitioning into saturated granular alluvium only 25' below city streets and buildings. High potential for subsidence in mixed-face transition zone

◆ Solution:

◆ HBI worked with the tunneling contractor to develop a Compaction Grouting Program to compensate for settlement over the TBM, to allow tunneling to progress through this zone without surface disturbance.

Mississippi Ave. Outfall Geologic Profile

I-25



Earth-pressure-balance TBM Portal



Grouting Holes

- Flush-joint 2-5/8" grout casing advanced to point 10' above tunnel crown on 10' centers, ahead TBM advance.
- Low slump compaction grout injected as tail shield passes by to compensate for soil loosening.

Compensation Grouting Plan

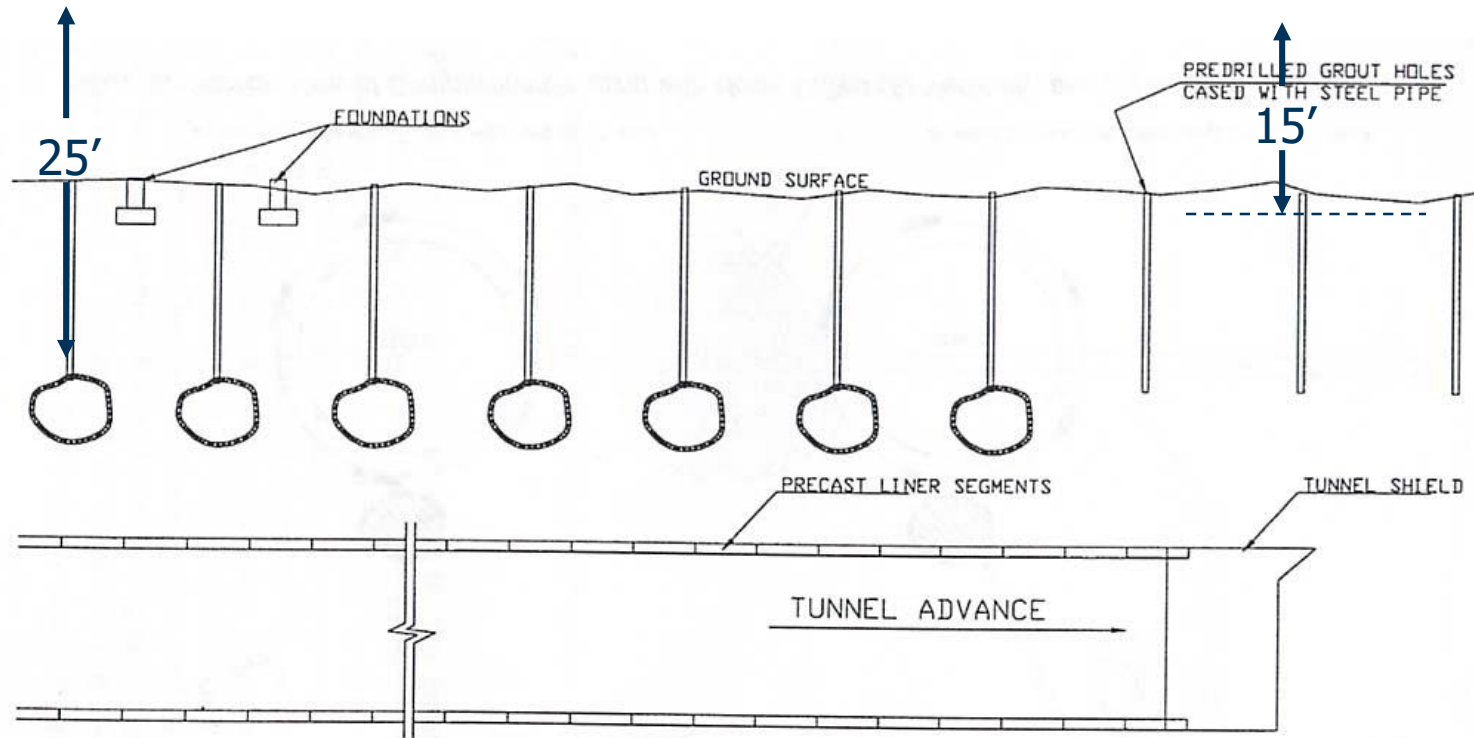


Figure 2-3 from *Practical View of Tunneling*, Grouting underground structures, used with permission of Soft-Ground Tunnel

Drilling and setting grout casing ahead of advancing TBM on tunnel centerline. Angled hole to avoid high pressure gas line.



Hp Gasline



Compensation Grouting at Transition Zone

- ◆ Extensometers and tell-tails showed when and where settlement occurred
- ◆ Compaction grouting commenced immediately to compensate for soil loosening above tunnel
- ◆ 6 cubic yards injected into three holes in transition zone at low pressures during tunneling
- ◆ Second phase of compaction grouting conducted later to tighten up ground in the transition area; higher pressures, lower takes

Sinkholes above crown of TBM



Drilling above tunnel to compaction grout voids



Grouting Holes in Sinkhole area

- grout casing advanced to point 5' above tunnel crown into center of each sinkhole, and at all stations where TBM parked between shifts. Voids logged and casing set for grouting
- Low slump compaction grout injected to fill and compact voids.
- Up to 5cu.yd placed in base of sinkhole areas at 150-200psi.
- Sinkhole filled and stabilized from bottom to surface under street.
- 78 in. brick-lined storm sewer also stabilized

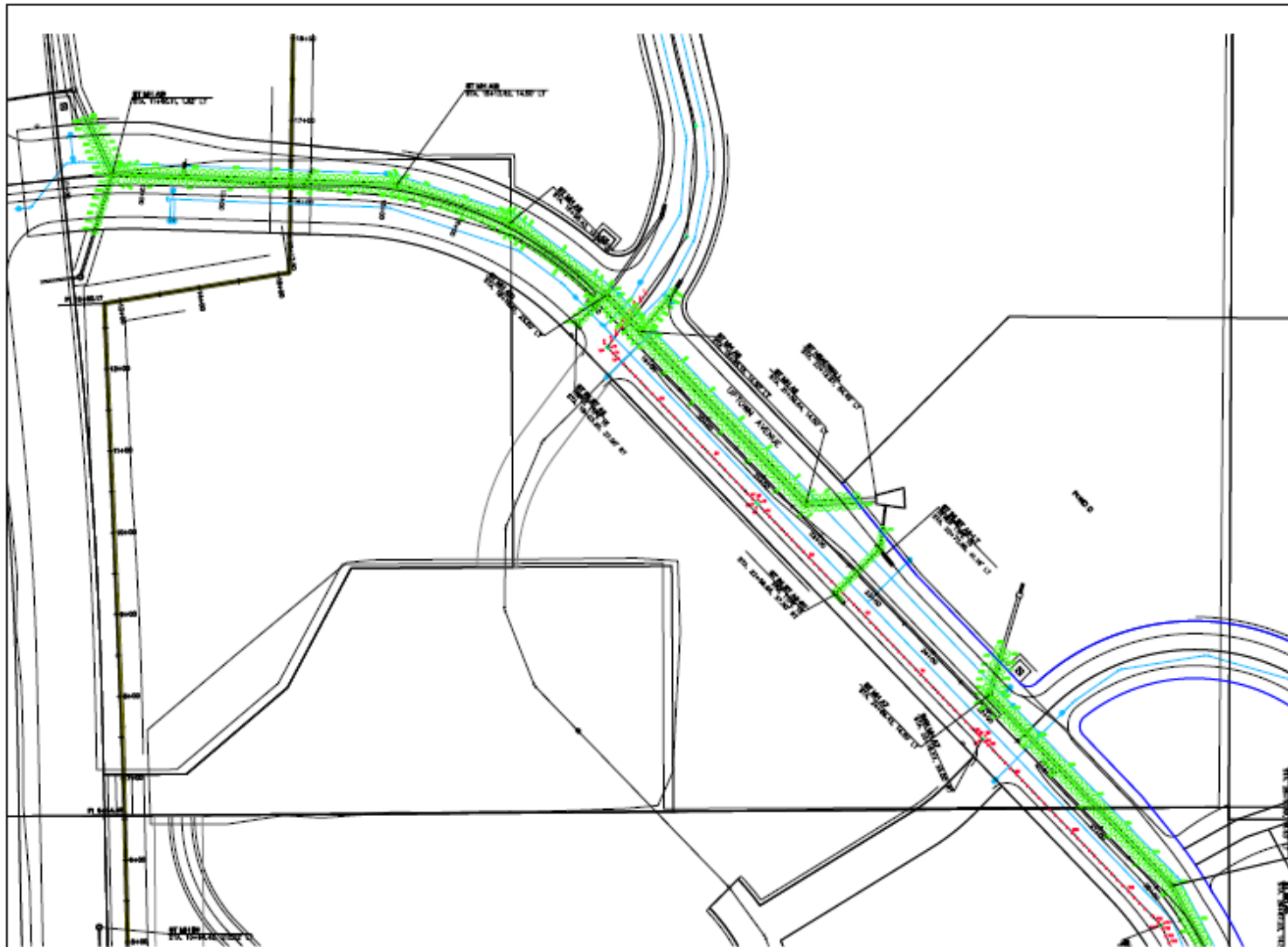
Project: Trench Backfill Rehab

Infrastructure Developed in 2005 – 2006

➤ The Issue

- Significant pavement distress & settlement in roadways starting in 2007 – 2008
- Settlement on the order of 3 – 6 inches with multiple areas exhibiting settlement as much as 12 to 24 inches
- CTL | Thompson determined that settlement was due to poor placement and compaction of sanitary and storm sewer trench backfill
- About 7600 lineal feet of sanitary and storm sewer trench and associated manhole backfill was at risk or exhibiting settlement
- Backfill depth ranged between 13 to 19 feet for sanitary sewer trench
- Storm sewer pipe diameter ranged from 24 to 78 inches. Backfill depth ranged between 9 to 22 feet with isolated areas of 25 to 30 feet depth.

▪



LEGEND:

- INDICATES COMPACTION SPOT LOCATION FOR SANITARY STORM TRENCH (SAGRELL) HOLES SHOULD BE 1.0 FEET OFFSET FROM CENTER LINE OF PIPE ALTERNATE SPOT AND LEFT OF CENTER LINE. PROPOSED SPOTS ARE PROVIDED IN SHEET 1. FIELD VERIFY BEFORE PULL TO GRADING.
 - INDICATES COMPACTION SPOT LOCATION FOR STORM SEWER (SAGRELL) SAGRELL LOCATION. SPOTS INDICATED APPROXIMATELY 1.0 FEET FROM EDGE OF PIPE FOR PIPES WITH DIAMETERS OF 30 INCHES OR LARGER. PIPES OF 48 INCHES OR LARGER SPOTS SHOULD BE 1.5 FEET OFFSET FROM COVER LINE OF PIPE. FIELD VERIFY BEFORE PULL TO GRADING.
- NOTES:
1. PRIOR TO AND AFTER GRADING, SANITARY AND STORM PIPES NEED TO BE SCOURED AND A HOLE SHOULD BE PROVIDED.
 2. UPSTREAM STORM PIPE NEEDS TO BE IMPROVED BY SCOURING PIPES PRIOR TO GRADING AT MANHOLE STAKE #1, #2, #3, #11, #12 IN UPSTREAM RANGE AND STAKE #3 THRU #6 IN JUNCTION PLACE. GRADING BELOW PIPE SHOULD BE DONE PRIOR TO GRADING SAGRELL ABOVE PIPE.
 3. TRAFFIC CONTROL IS THE RESPONSIBILITY OF CONTRACTOR. TRAFFIC CONTROL PLAN NEEDS TO BE PROVIDED TO CLIENT AND CITY OF BROWNFIELD FOR APPROVAL.
 4. ADJUSTMENT OF LOCATION, DEPTH AND GREAT WALLING MAY BE MADE BY THE CONTRACTOR DEPENDING UPON FIELD CONDITIONS.

ARISTA	
Roadway - Utility Trench Compaction Grouting Locations	
<small>CYL/THOMPSON, INC. CONSULTING ENGINEERS 101 WEST 4TH STREET, SUITE 200, BROWNFIELD, LA 70513-1011</small>	
<small>DRAWN BY: JH CHECKED BY: MJC</small>	<small>DATE: 11/16/19 PROJECT NO: 2018-07-108</small>
<small>ISSUED BY: MJC</small>	<small>REVISION: 11/16/19 SHEET: 01</small>



Photo 1
June 4, 2009
Project No. DN 43,846-145



Photo 2
June 4, 2009
Project No. DN 43,846-145



Photo 355
June 8, 2009
Project No. DN 43,846-145



Photo 356
June 8, 2009
Project No. DN 43,846-145



Photo 358
June 8, 2009
Project No. DN 43,846-145



Photo 63
June 5, 2009
Project No. DN 43,846-145



Photo 64
June 5, 2009
Project No. DN 43,846-145



Photo 66
June 5, 2009
Project No. DN 43,846-145



Photo 81
June 5, 2009
Project No. DN 43,846-145

Remediation Alternatives

- ◆ Mechanical Remove, Rework or Replace of Existing Fill
- ◆ Densify the Existing Fill in Place using Low Mobility Compaction Grouting Techniques

Considerations for Remediation

◆ Mechanical Methods

- Total Trench Remediation 7600 lineal ft
- Top of trench may need to be on the order of 12 to 18 feet to address backfill zone
- Depth of trench may range between 6 to 22 feet
- Shoring and/or bracing of trench sides to protect existing improvements
- Underpinning & support of shallow utility crossings
- Difficult to address fill beneath pipe at gradient/ manhole changes
- Portions of existing fill too wet to remove & replacement may need to dry or export/import
- Prolonged lane and road closures

Considerations for Remediation

◆ Mechanical Methods (cont'd)

- Weather could impact progress
- Significant cut & removal of existing pavement system and associated, curb-gutter & landscape islands
- Schedule/duration
- Cost estimate varied from \$2.5 to 3.5 million
- If done newly placed & compacted backfill may experience consolidation & settlement

Considerations for Remediation

◆ Compaction Grouting Techniques

- Method uses a 3 inch diameter core for access through pavement
- No need to remove & replace pavement
- Weather typically does not impact contractor's operations
- Increasing density in place – Alternating patterns allows verification of previously grouted areas
- Can potentially lift & correct grade
 - Concrete Elements
 - Asphalt Elements
- Void fill beneath concrete elements with either grout or polyurethane
- Existing utility avoidance

Considerations for Remediation

- ◆ **Compaction Grouting Techniques (cont'd)**
 - Minimal disruption to traffic flow
 - Traffic lanes re-open each night & on weekends
 - Design/build approach allows field adjustment as conditions warrant
 - Address fill beneath pipe and gradient drops
 - Rotomill & repave as needed – no complete removal necessary except at previously damages areas



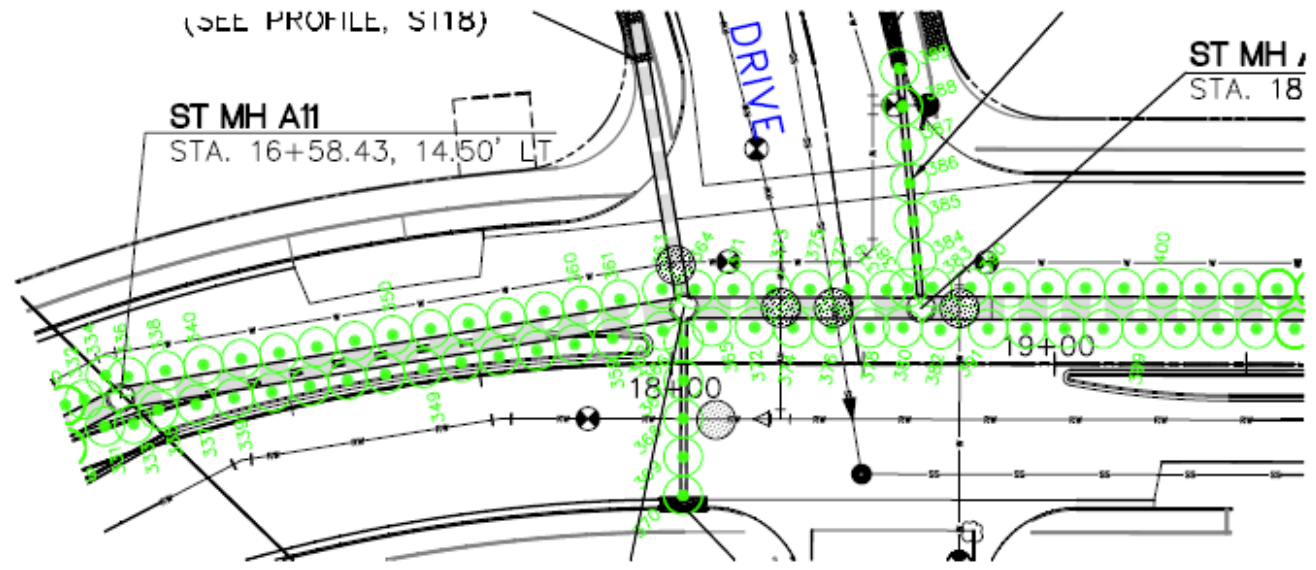
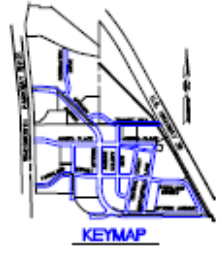
2011.8.2 10:05 AM



2011.8.3 11:51 AM



2011.8.3 12:42 PM



CTL THOMPSON
ENGINEERS
1871 W. 12th AVENUE, DENVER, CO 80204
PHONE (303) 834-2777
www.ctlthompson.com

Afsta
Roadway - Utility Trench
Compaction Grouting
Locations

CTL PROJECT NO.
SNA6267148
DATE
11/02/2010

ST-6A



2011.9.1 10:36 AM



2011.9.1 02:36 PM



2011.9.23 09:06 AM



2011.9.7 10:04 AM



BROOMFIELD LN

2011.11.9 11:05 AM



SPEED
LIMIT
20

2011.9.23 02:06 PM



2011.9.23 12:01 PM

ARISTA
Roadway Monitoring

November 27, 2012				April 3, 2014				
PT #	HI	Minus	Elevation	PT #	HI	Minus	Elevation	Diff.
1	5483.859	3.865	5479.994	1	5483.747	3.760	5479.987	-0.007
2	5483.859	4.295	5479.564	2	5483.747	4.193	5479.554	-0.010
4	5473.687	4.305	5469.382	4	5472.130	2.752	5469.378	-0.004
5	5473.687	5.875	5467.812	5	5472.130	4.323	5467.807	-0.005
6	5473.687	12.300	5461.387	6	5466.319	4.922	5461.397	0.010
11	5447.040	7.840	5439.200	11	5445.661	6.452	5439.209	0.009
12	5447.040	8.825	5438.215	12	5440.233	2.018	5438.215	0.000
30	5446.642	3.393	5443.249	30	5447.752	4.498	5443.254	0.005
31	5446.642	3.198	5443.444	31	5447.752	4.303	5443.449	0.005
32	5446.642	2.862	5443.780	32	5447.752	3.978	5443.774	-0.006
39	5483.859	2.210	5481.649	39	5483.747	2.110	5481.637	-0.012
40	5483.859	2.720	5481.139	40	5483.747	2.618	5481.129	-0.010
41	5483.859	3.968	5479.891	41	5483.747	3.860	5479.887	-0.004
42	5483.859	4.657	5479.202	42	5483.747	4.540	5479.207	0.005
86	5446.642	2.640	5444.002	86	5447.752	3.748	5444.004	0.002
87	5446.642	1.795	5444.847	87	5447.752	2.905	5444.847	0.000

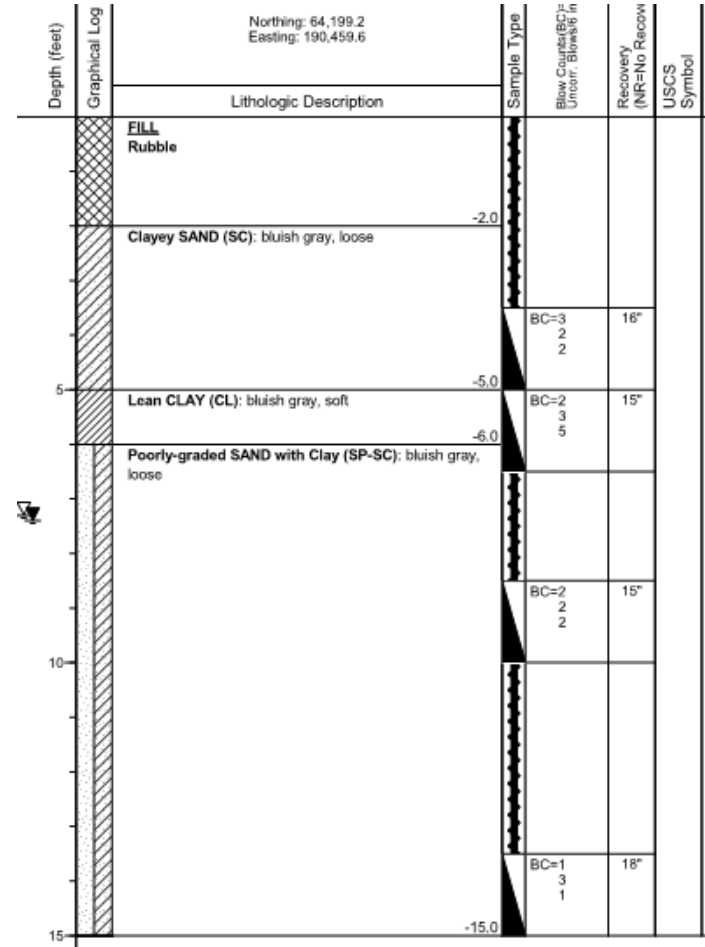
Compaction Grouting Project Summary

- ◆ A total of 1800 grout locations
- ◆ Total grout volume:
 - 508 cu yds sanitary sewer
 - 1214 cu yds storm sewer
- ◆ Estimated volume replacement ranged between 8 to 12 percent
 - Translates to increase density in that same range
- ◆ Cost at budget due to the design build approach adjustments allowed additional areas to be treated which were not part of the original scope
- ◆ Minimal impact to the public
- ◆ Essentially no additional trench/pavement settlement

Ground Modification Techniques: Increase Bearing Capacity / Reduce Settlement

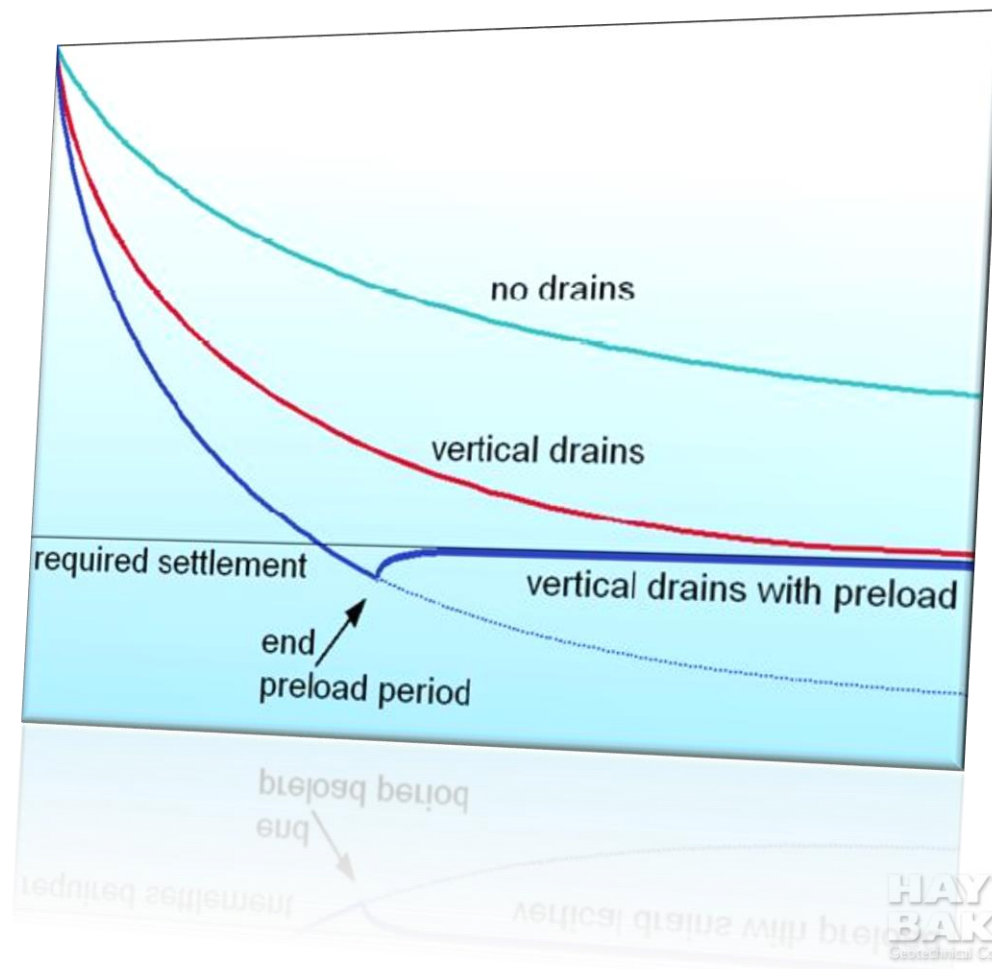


Soft Soils Under Proposed Exit Ramp

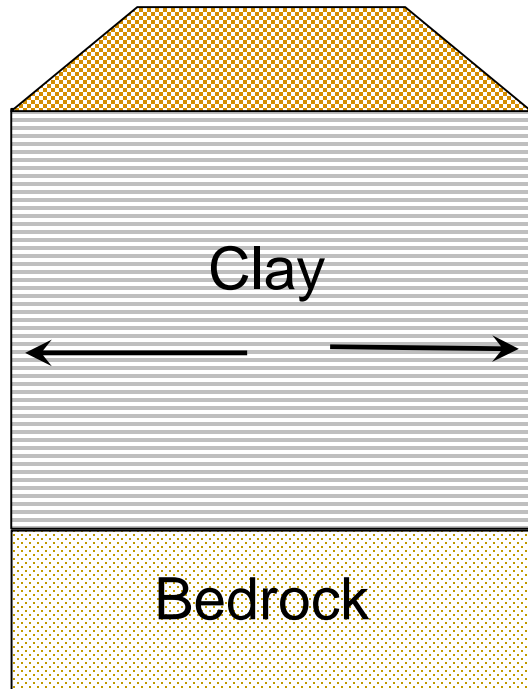


Wick Drains: Decrease Settlement

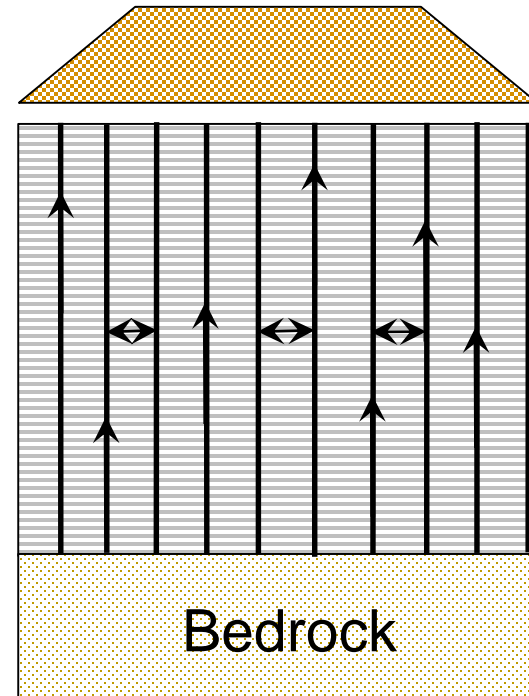
- ◆ Prefabricated Vertical Drains (PV drains, PVD's)
- ◆ Wick Drains
- ◆ Synthetic drains
- ◆ Band Drains
- ◆ Strip Drains

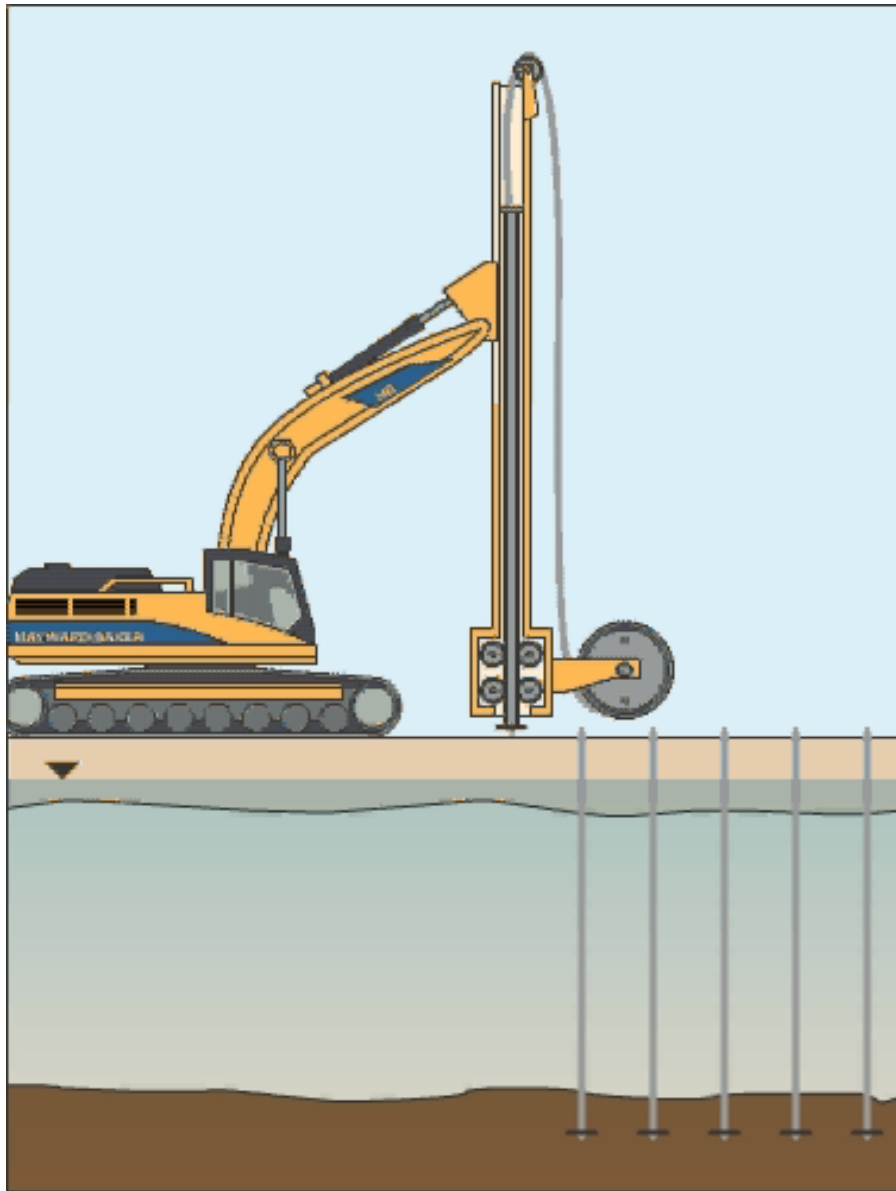


Wick Drains



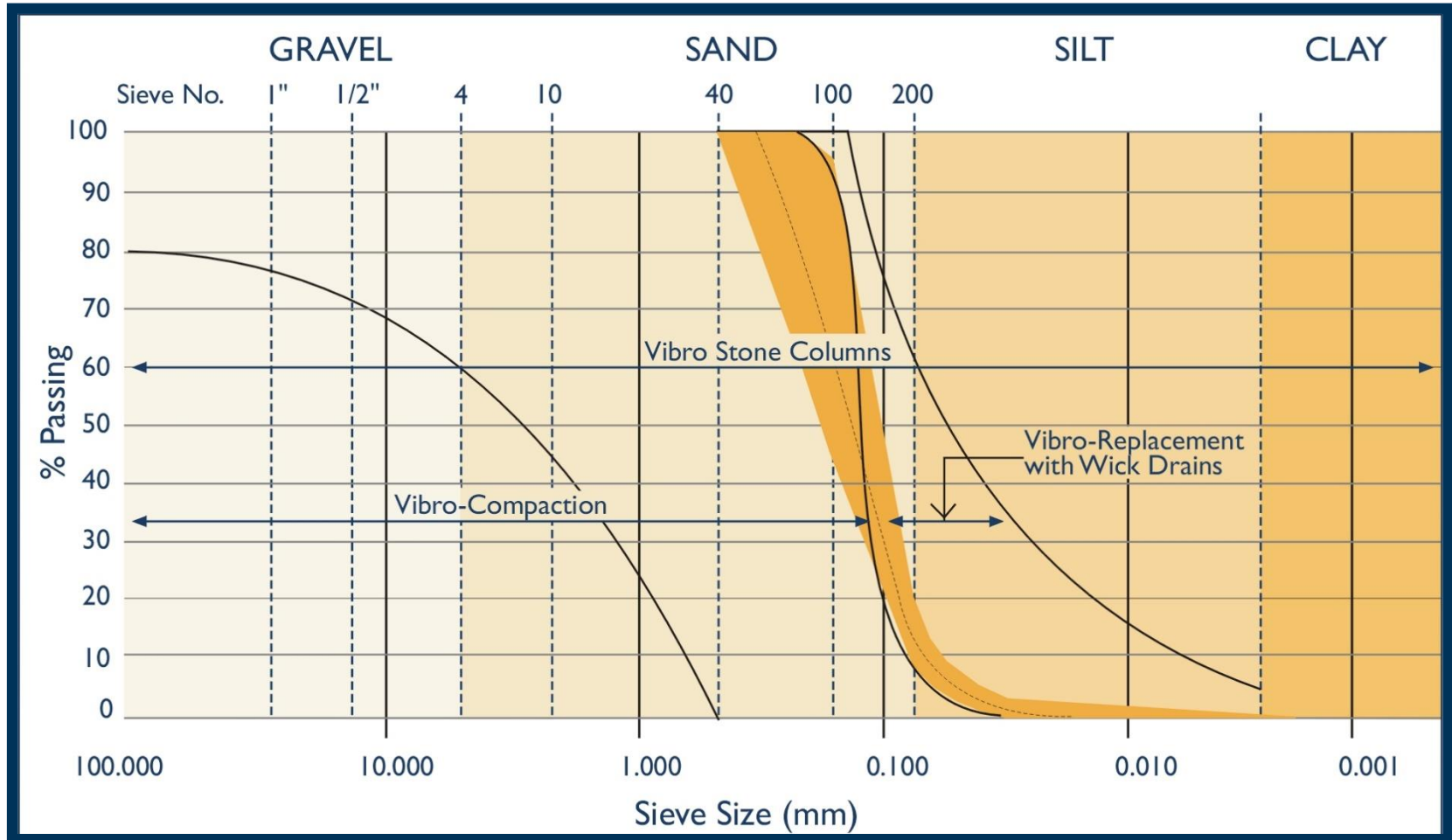
With Drains







No Time for Settlement: Vibro Techniques



Vibro Piers – Time Tested Technique

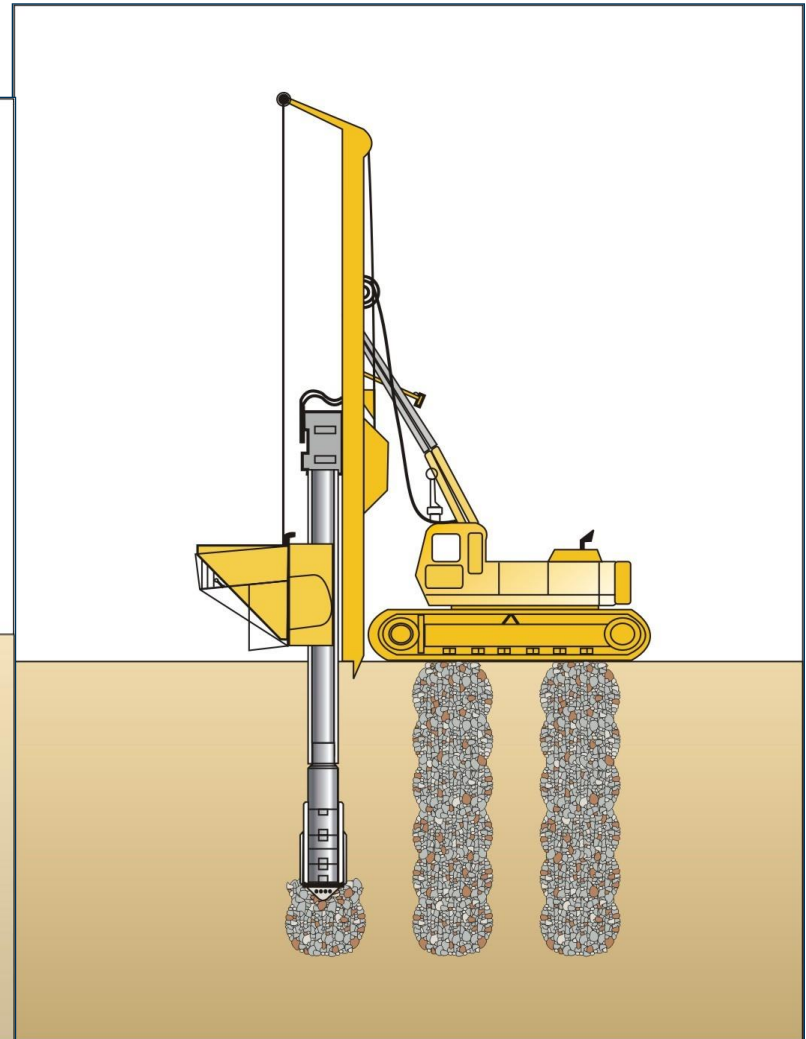
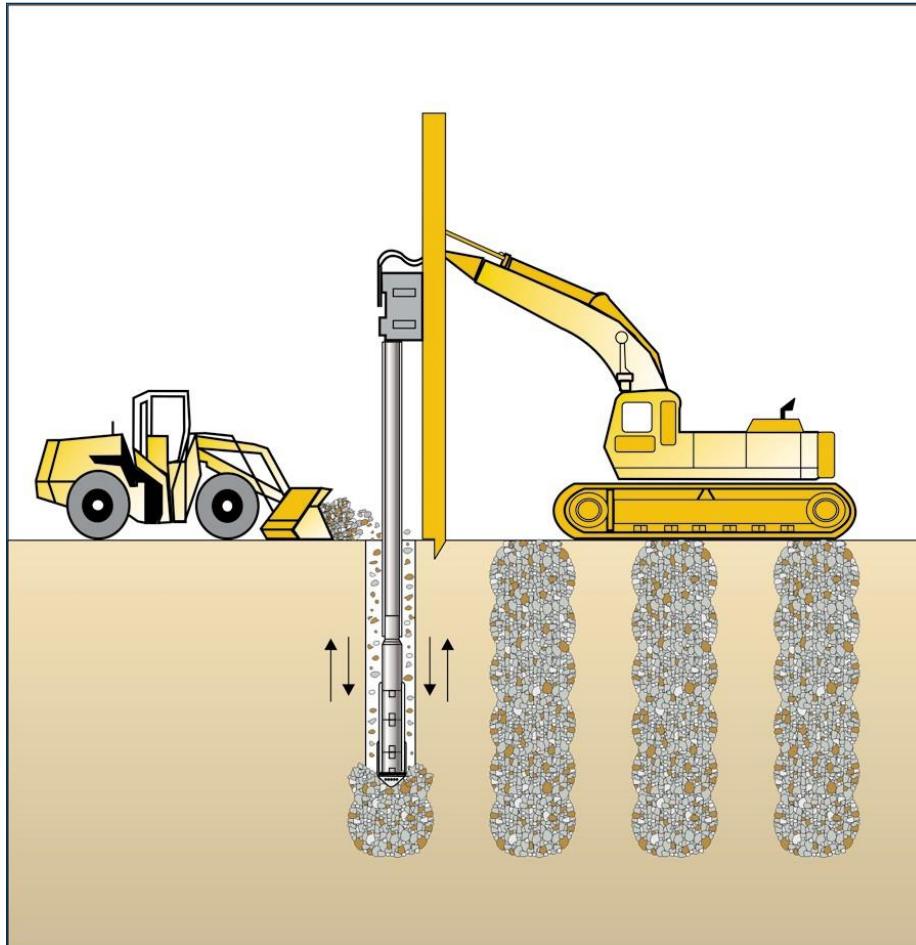


Keller rigs, circa 1932

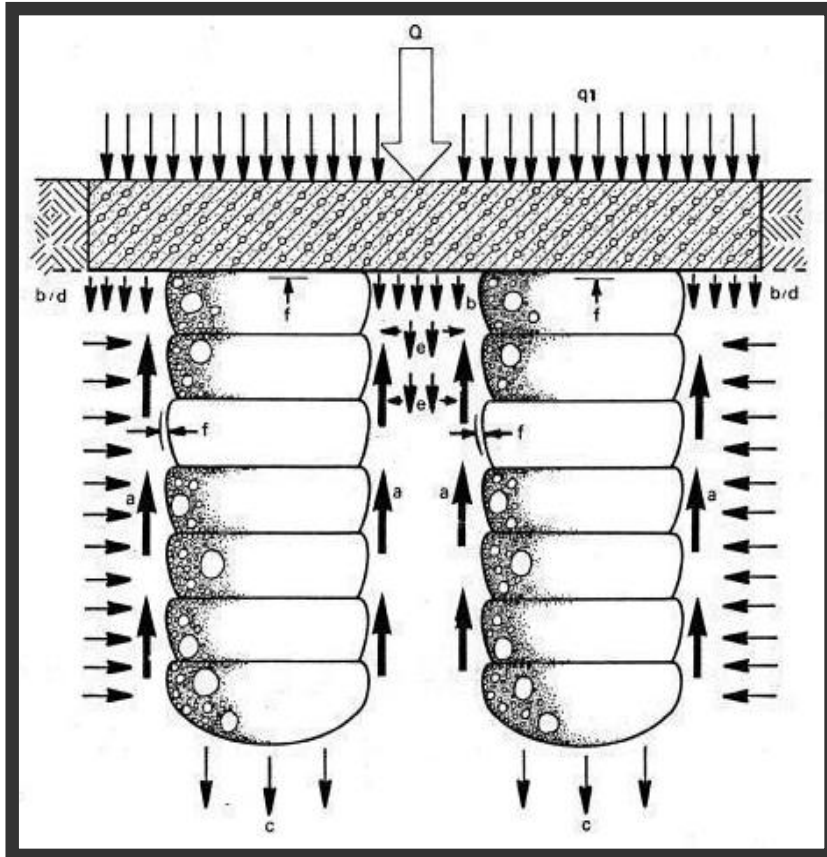
Keller rig crews, at 1910
50th anniversary



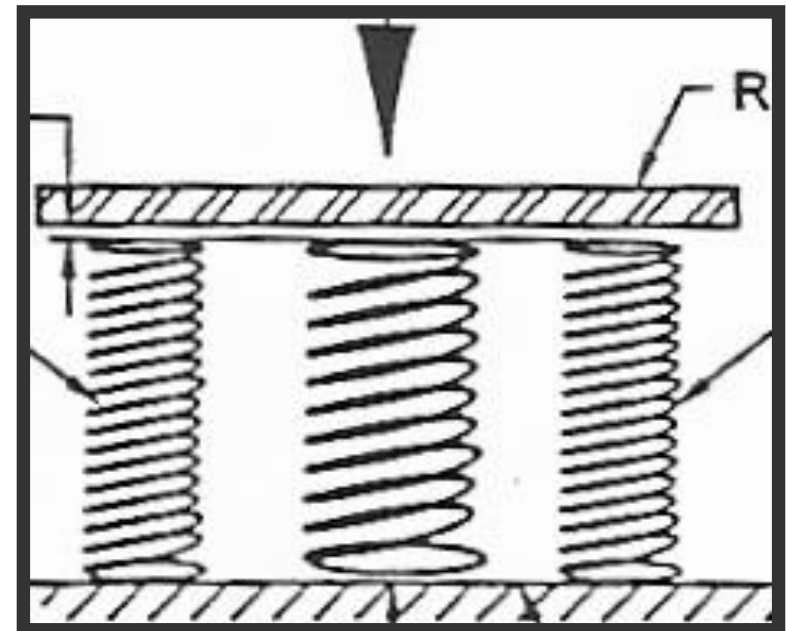
Installed using top or bottom feed methods (Above or Below Water Table)



Stone Columns/Aggregate Pier Design Methodology



Reinforcement
Mechanism / Model



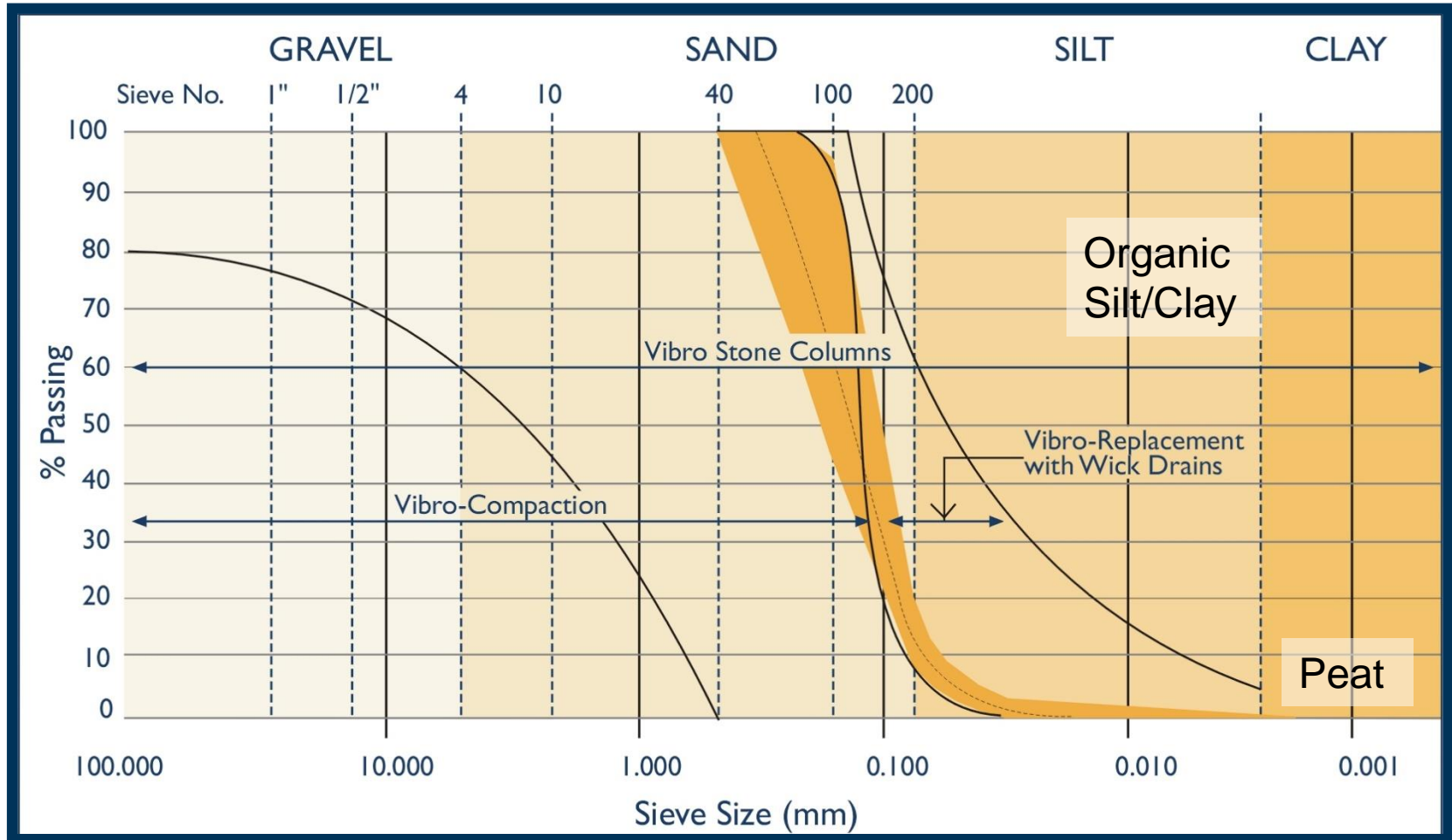
Vibro Piers for MSE Wall Support



Vibro Piers: Bottom Feed Unit



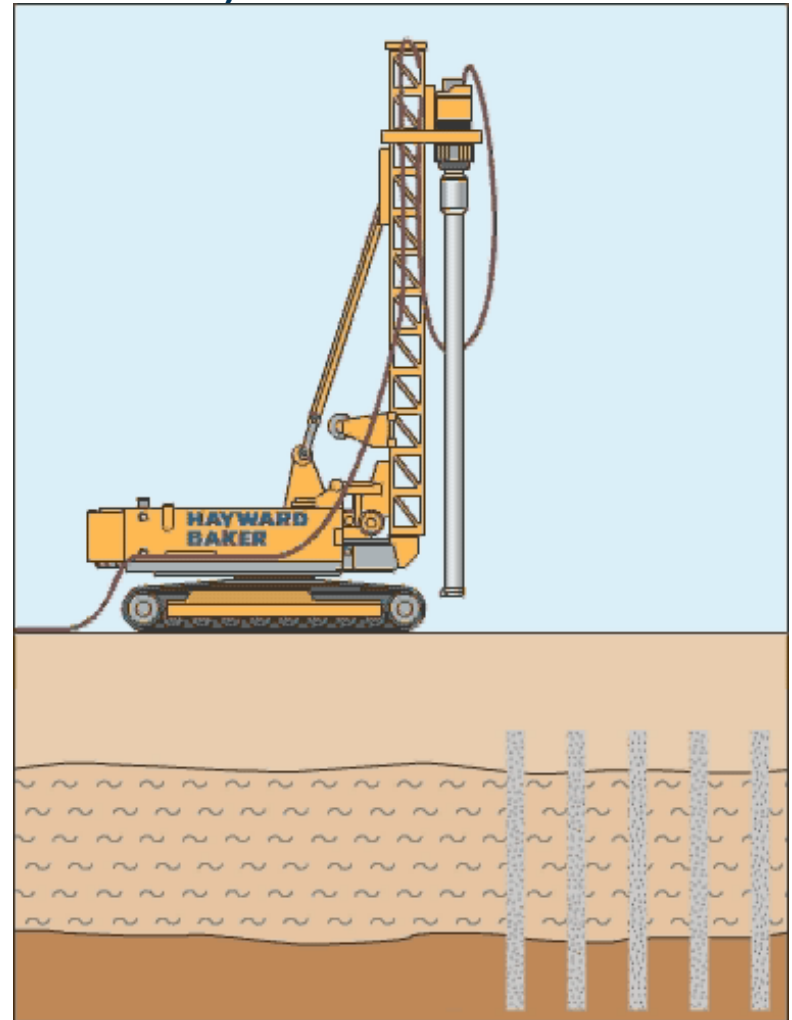
What if the Ground is Very Soft!



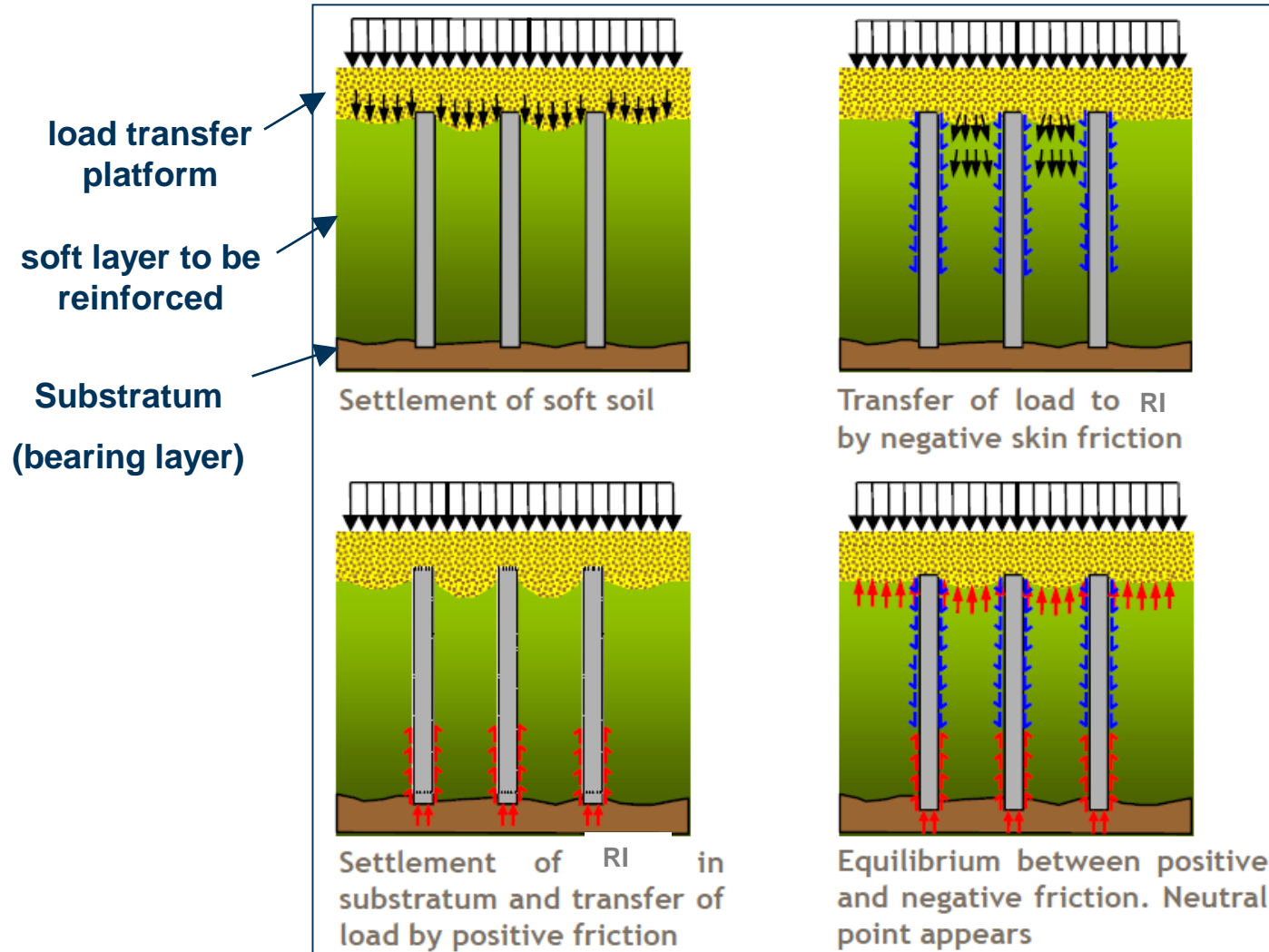
Rigid Inclusions

(controlled modulus columns)

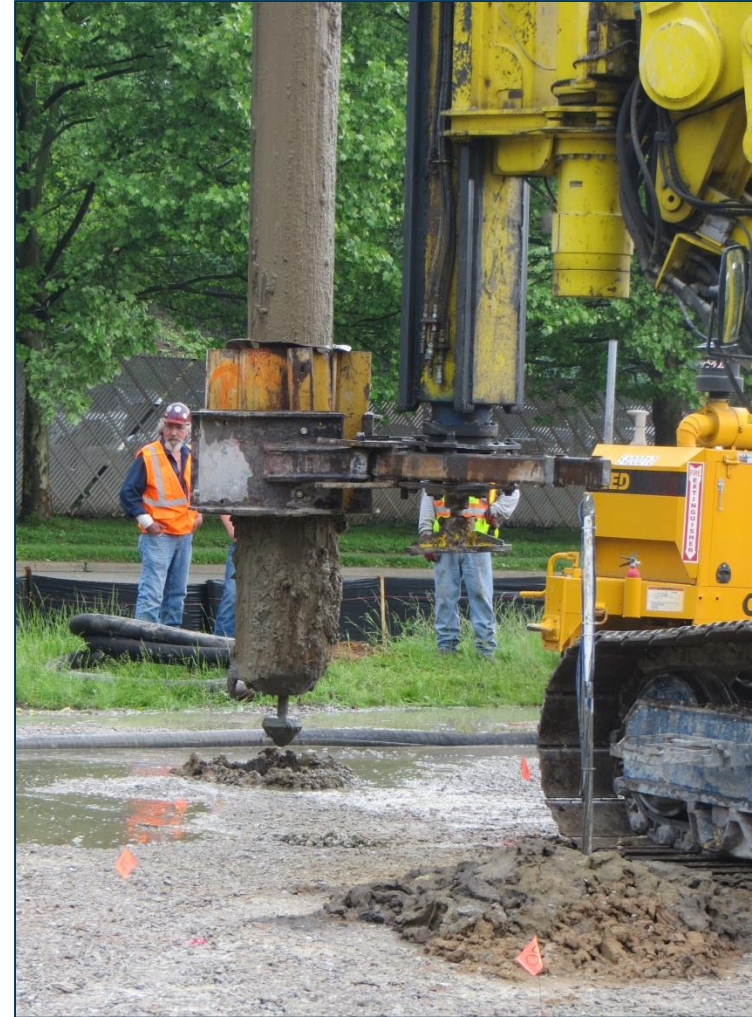
- ◆ High modulus columns constructed through compressible soils
- ◆ Not in direct contact with foundations
- ◆ Used to reinforce soft, compressible soils
- ◆ Load transfer platform

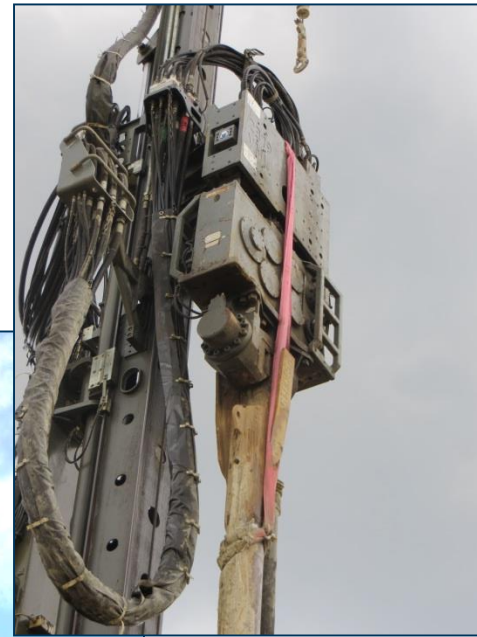


Subsurface Load Transfer



Rigid Inclusion Construction Methods



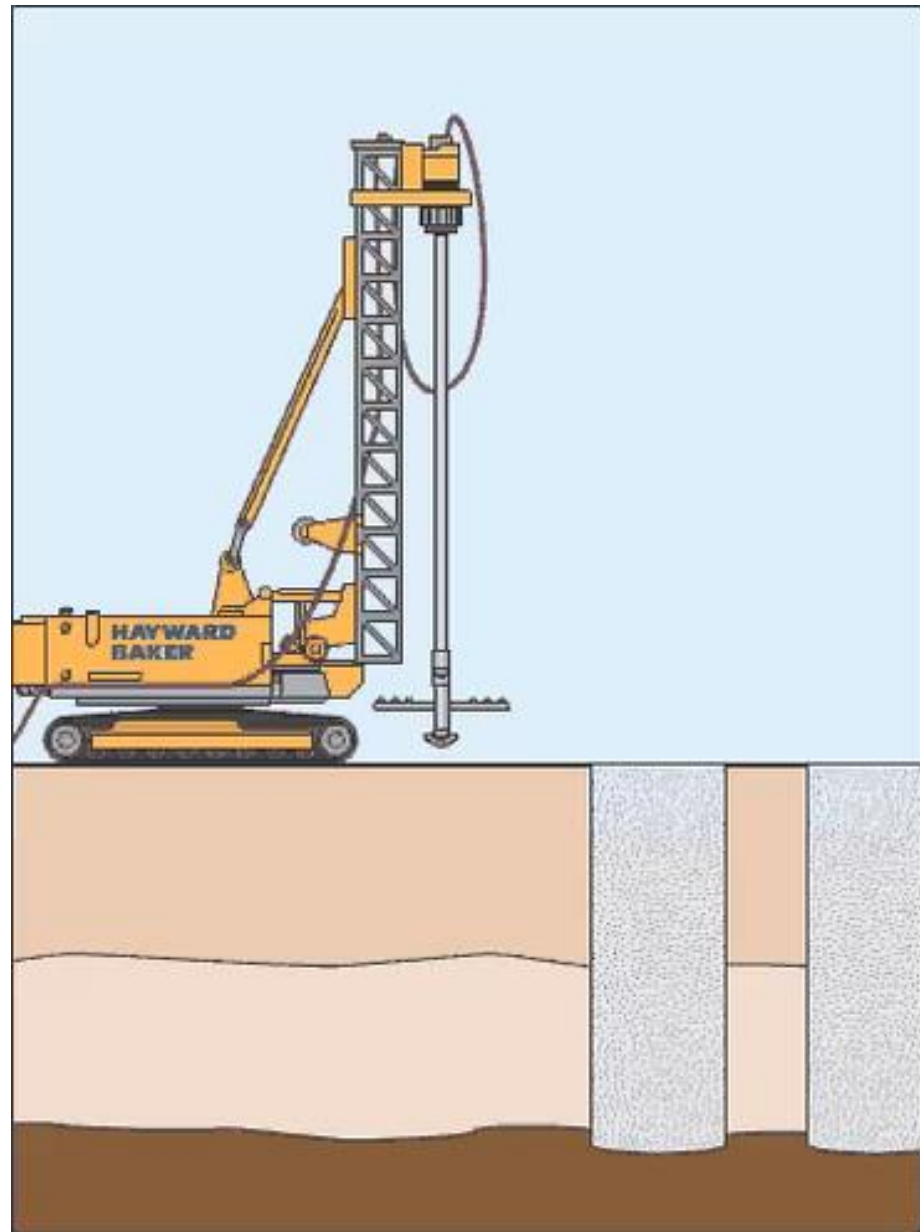


Still too much calculated settlement? Soil Mixing



Soil Mixing

- ◆ 50psi to 500psi soilcrete
- ◆ Dry or Wet Mixed
- ◆ Data Acquisition Available

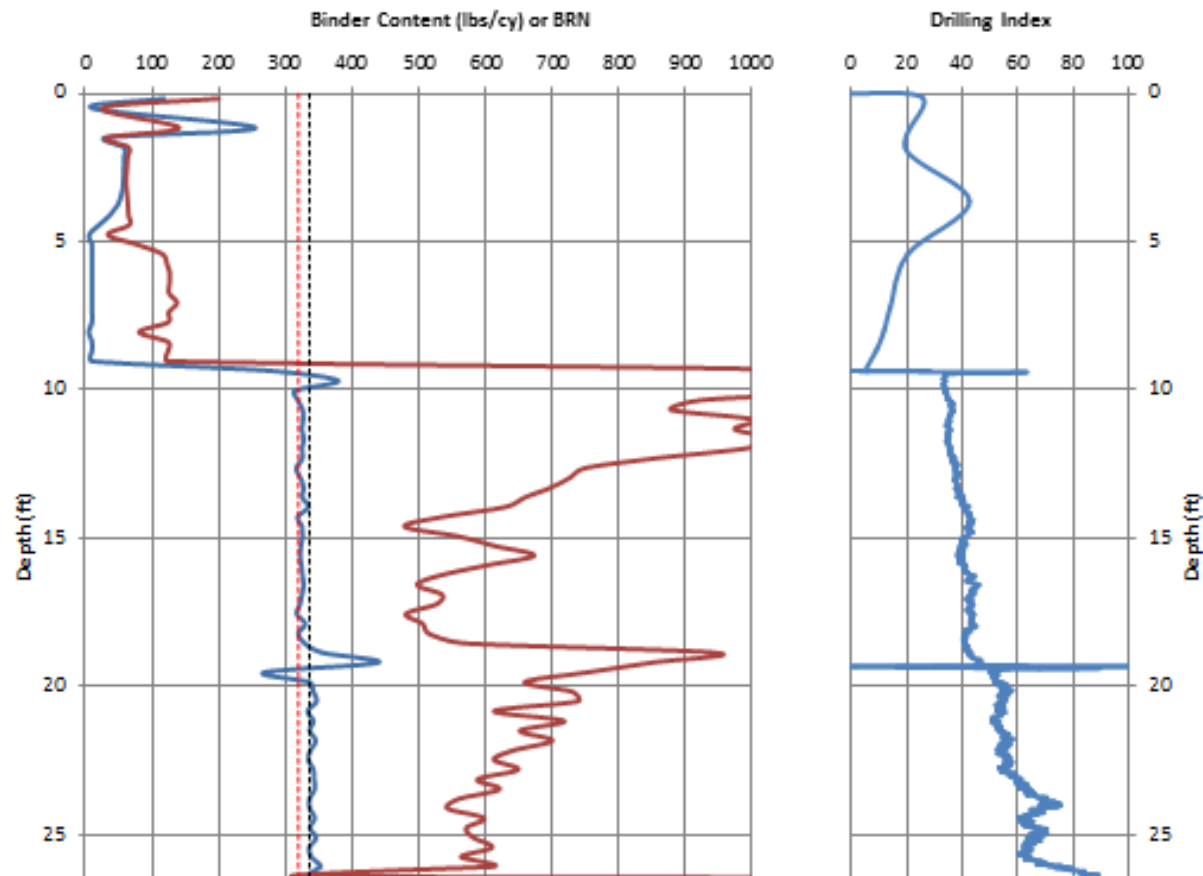


Data Acquisition for Soil Mixing (HBIDAQ)



West Dowling Phase 2 Wet Soil Mixing Installation Log Column 1-184

Column Diameter: 8.0 ft	Penetration Avg. Injection Rate: 155.06 gpm	Date: 7/14/2014
Column Length: 26.58 ft	Withdrawal Avg. Injection Rate: 39.50 gpm	Start Time: 2:54 PM
Top of Column Dep: 9.34 ft	Avg. Penetration Rate: 0.99 ft/min	End Time: 3:18 PM
Average Grout SG: 1.44	Avg. Withdrawal Rate: 9.03 ft/min	Penetration: 00:20:04
Total Grout Volume: 3048 gal	Inclination (X): -0.58°	Withdrawal: 00:03:42
Total Binder Weight: 16399 lbs	Inclination (Y): 0.43°	Total Time: 00:23:46
Avg. Binder Content: 347 lbs/cy	Project X: 0.00 ftUS	Rig: BG-28
Job Number: 860047-28	Project Y: 0.00 ftUS	Project Z: 0.00 ft



Soil Mixing at Work



Mass Soil Mixing

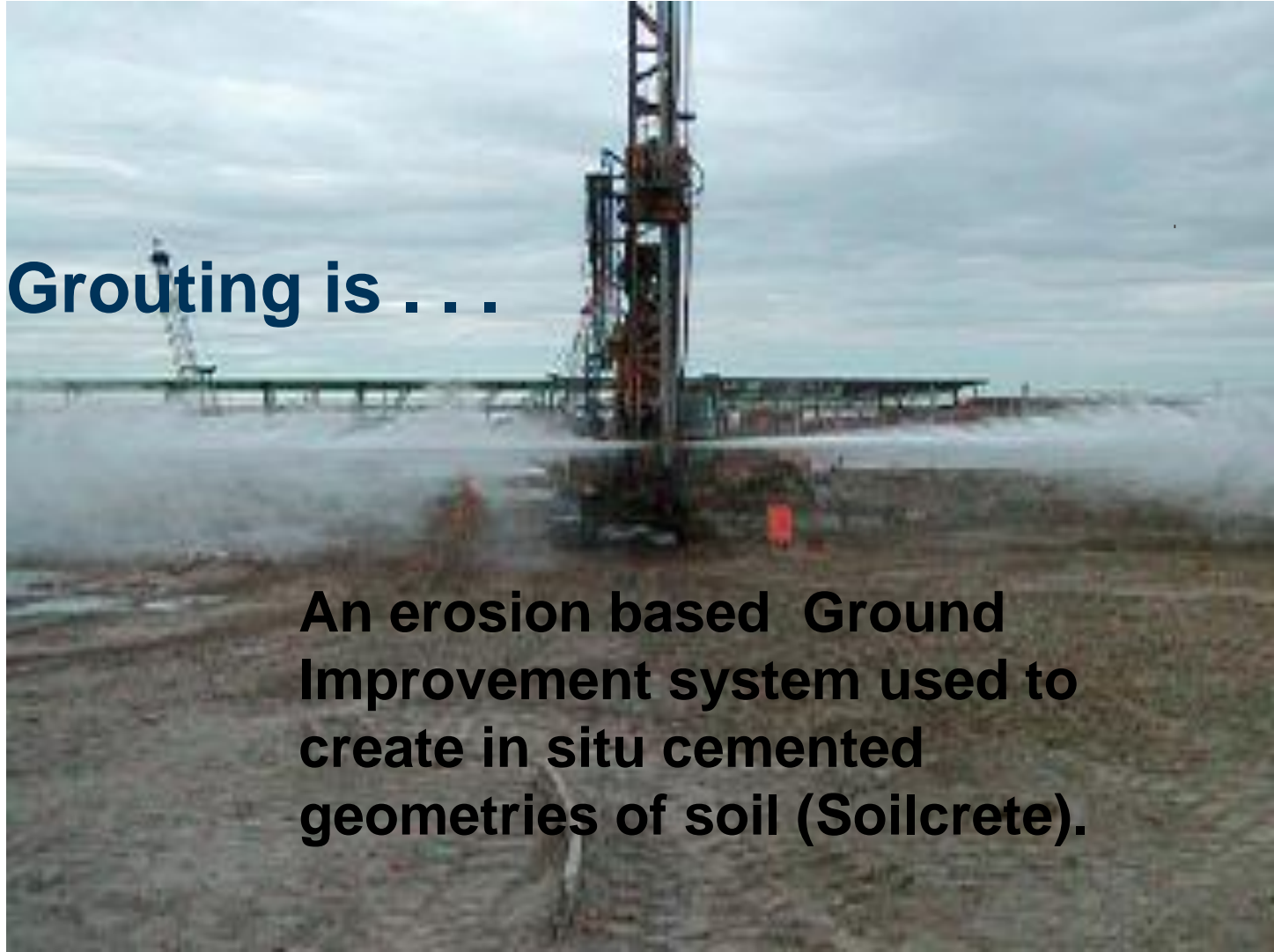


What if access is a problem? Jet Grouting



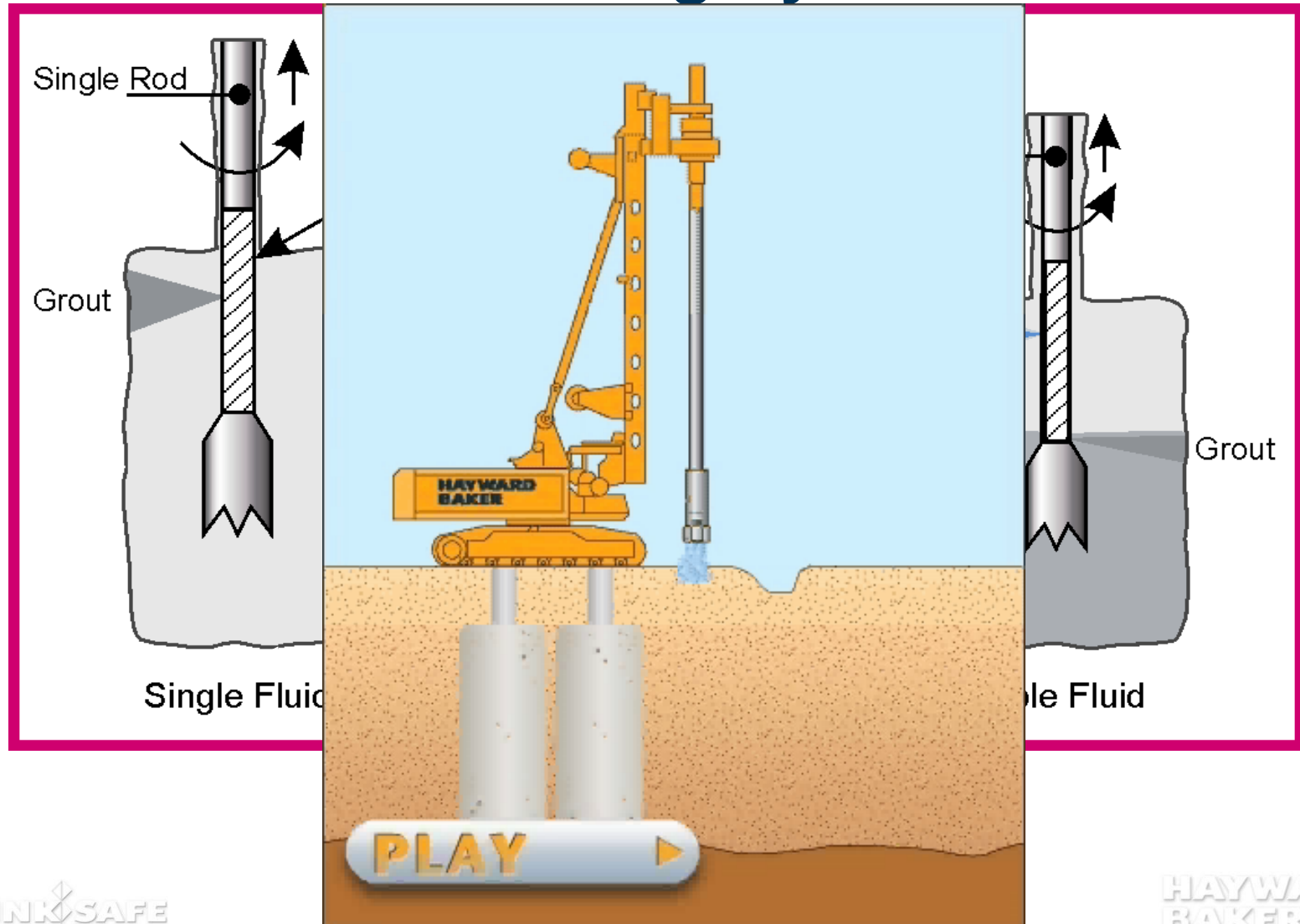
Jetting for Scour Protection: Ortega Hwy 74 Bridge

Jet Grouting is . . .



An erosion based Ground Improvement system used to create in situ cemented geometries of soil (Soilcrete).

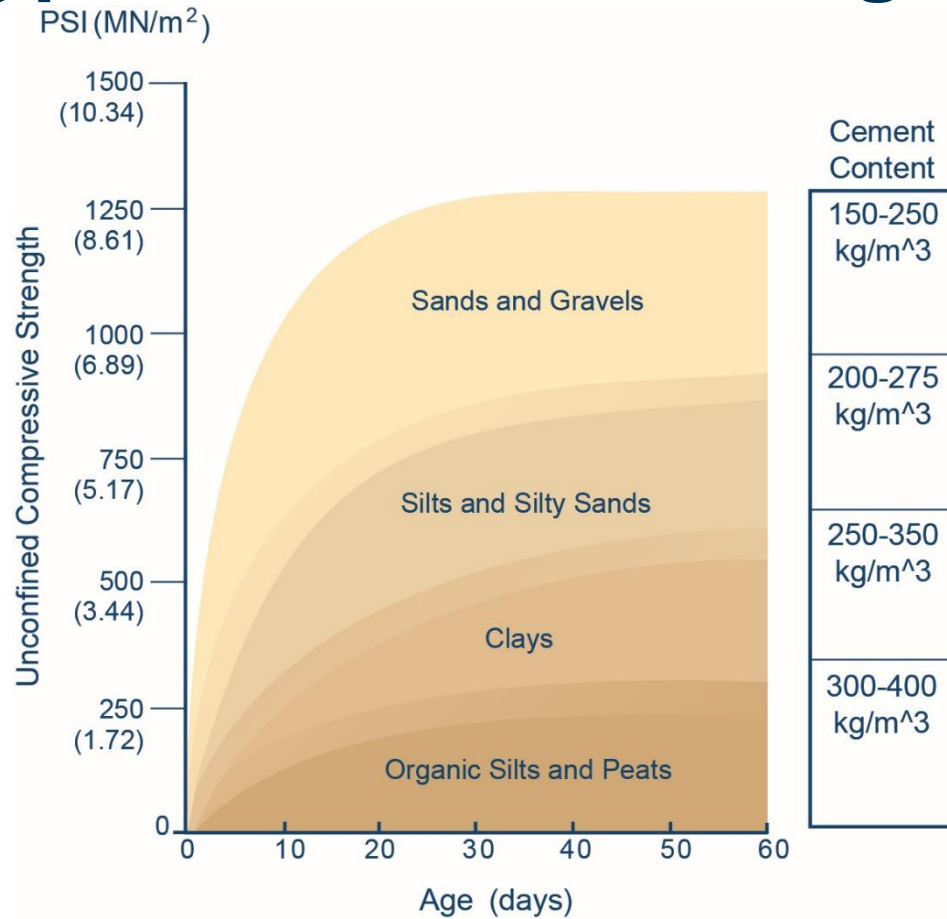
Jet Grouting Systems



Geometry is a function of:

- ▶ Dynamic pressures
 - Velocity of the fluid
 - Air shroud (reduces attenuation)
 - Nozzle focus and condition
 - Injection volume rate
- ▶ Erosion time
 - Rotation speed
 - Lift speed (step time and height)
- ▶ And most importantly: Soil strength and plasticity

Typical Soilcrete Strengths



Questions?

